



Parole Tributary to Church Creek Hydrologic and Hydraulic Analysis Report

Prepared for:
Anne Arundel County

Final

Date: November 30, 2017

LimnoTech 
Water | Scientists
Environment | Engineers

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1 Introduction

As part of its National Pollutant Discharge Elimination System (NPDES) Municipal Separate Storm Sewer System (MS4) permit, Anne Arundel County is required to conduct long term biological, chemical, and physical monitoring of two developed sites. In 1998 the Church Creek watershed was selected as one of these permanent monitoring sites.

The Church Creek watershed is highly developed and contains a large percentage of impervious surface. The watershed is mostly commercial and includes several large developments, including the Annapolis Towne Centre, Festival at Riva Shopping Center, and the Annapolis Harbour Center. The watershed has been undergoing redevelopment, including the reconstruction of Parole Plaza into the Annapolis Towne Centre, which has led to the addition of various BMPs throughout the site.

The purpose of this report is to determine the effect of the 1-, 2-, 10-, and 100-year design storms on the stream bed. This was done by using SWMM to model the hydrology of the watershed, and HEC-RAS to model the stream hydraulics. Key results of the modeling include stream flow, shear stress, and stream velocity. The modeling results were evaluated to determine the stability of the stream during the four modeled design storms.

Several previous studies have been performed on this site to determine the effectiveness of BMP implementation in mitigating peak flows in the watershed, including a 100-year floodplain study performed by McCrone in 2001 (McCrone, 2001), and a hydrologic and hydraulic analysis done by KCI in 2010 (KCI Technologies, Inc.). This report serves as a update to these reports, and includes both hydrologic and hydraulic analysis of the creek and its watershed. This report is also required to be included with Anne Arundel County's MS4 Annual Report.



2 Hydrology

The Church Creek watershed is generally sloped from north to south and conveys water through a well-developed, commercial area via overland flow and a storm system to Church Creek and its tributary, Parole Branch. After the confluence of these two streams, the water flows east and under Solomons Island Road through a 96" CMP culvert (see Figure 1).

The hydrologic modeling was done in PCSWMM using the 2014 impervious surfaces layer provided by the County and the current SSURGO soils layer as inputs (USDA 2016). The system was modeled conservatively in that runoff generated within the watershed flows directly to the outfall or BMP that captures it. Typically, a collection system is not designed to convey very large runoff events (e.g. 100-year event), and therefore water will pond locally before flowing to the outfall or stream. However, because the size and elevations for the storm system were not known, oversized pipes were modeled to convey the flows to each outfall and BMP. Because the model directs flow to the outfall or stream through an enlarged pipe, the resulting peak flows and intensities are likely higher than would be seen during an actual precipitation event. Although additional information on the storm system could be useful to further refine these results, this conservative approach is considered appropriate for this model application.

Forty-one (41) BMPs were identified in the Church Creek watershed (Figure 1). These BMPs were identified from either the KCI report completed in 2010, or the Anne Arundel County BMP Record Review and Update project currently being conducted by LimnoTech. Fourteen (14) of these BMPs were identified as having sufficient detention to impact runoff, and were included in the model. The remaining BMPs were either too small to affect the downstream hydrograph (e.g. single family housing infiltration practices) or were designed for water quality and not detention (e.g. Baysavers), and thus they were not included in the model.





Figure 1. Church Creek Existing Conditions

Of the 14 identified BMPs, six (6) are infiltration practices, four (4) are dry ponds, three (3) are wet ponds, and one (1) is a sand filter. Drainage areas for each modeled BMP were developed using a combination of GIS data including elevation files and storm drain profiles; previous delineations done by KCI in 2010; and plan documents received from the County through the Anne Arundel County BMP Record Review

and Update project. The modeled BMP locations, their drainage areas, and the Hydrologic Soil Groups (HSGs) are shown in Figure 2 below. The HSGs classifications are used to estimate the runoff potential of various soil types. Soils classified as HSG A soils have low potential for runoff, while soils classified as HSG D have high potential. As Figure 2 shows, most of the Church Creek Watershed is classified as HSG D and will generally produce higher runoff.

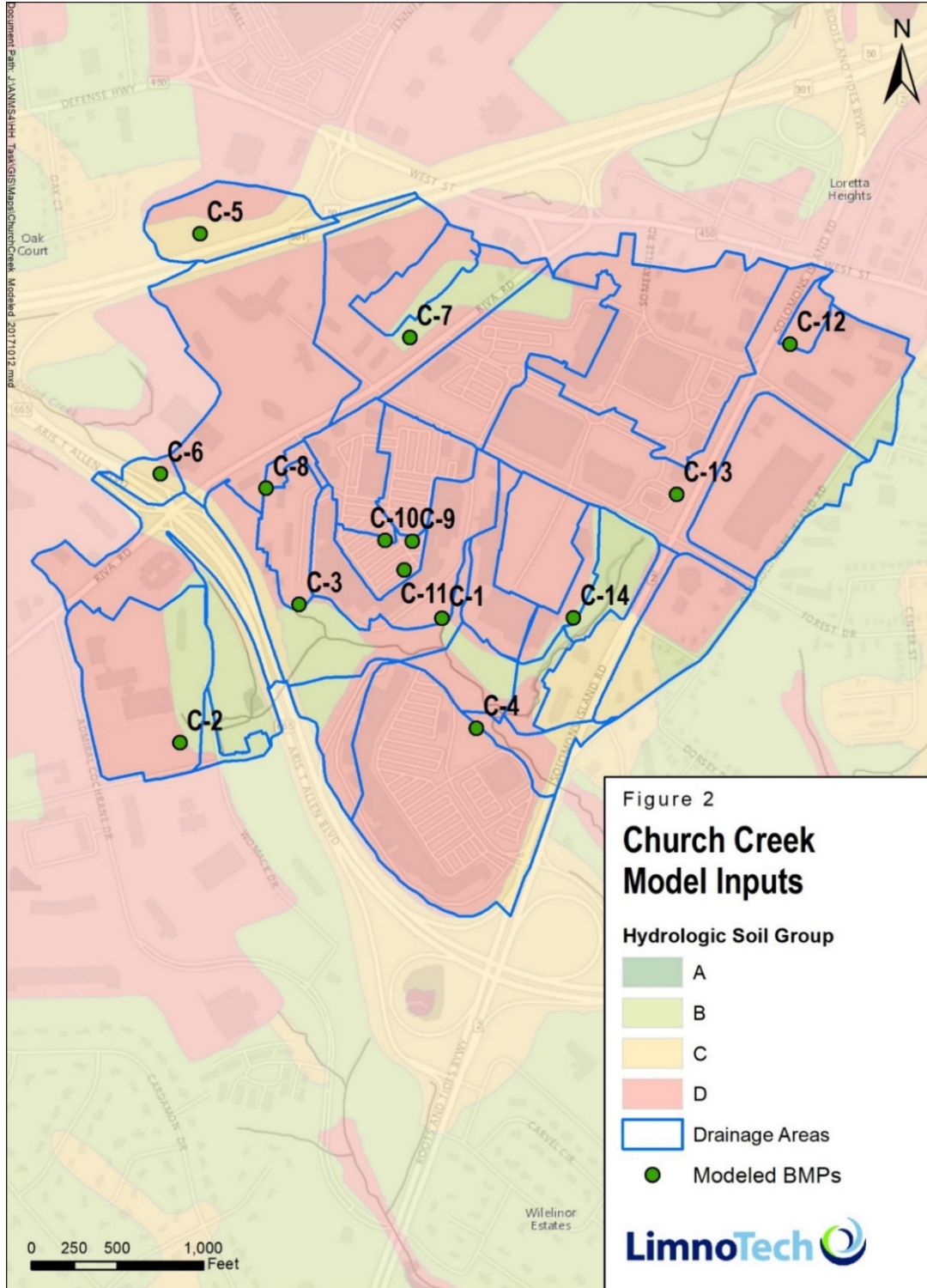


Figure 2. Church Creek Model Inputs



Available information varied for each BMP, so a breakdown of how each BMP was modeled is provided below:

BMP C-1: This wet pond was modeled as a storage unit and outfall using elevation-area and stage-discharge curves previously developed by KCI.

BMP C-2: Due to lack of information, this dry pond was assumed to treat the first 1" of rainfall based on stormwater regulations. This was done in the model by increasing depression storage of the subcatchment to 1".

BMP C-3: This dry pond was modeled using an elevation-area curve developed by KCI, and known culvert, emergency spillway, and overflow weir data from KCI that was verified through plans provided by the County.

BMP C-4: This wet pond was modeled as a storage unit and outfall using elevation-area and stage-discharge curves previously developed by KCI.

BMP C-5: This wet pond was modeled as a storage unit and outfall using elevation-area and stage-discharge curves previously developed by KCI.

BMP C-6: This infiltration BMP was modeled as a storage unit and outfall using elevation-area and stage-discharge curves previously developed by KCI.

BMP C-7: Due to lack of information, this dry pond was assumed to treat the first 1" of rainfall based on stormwater regulations. This was done in the model by increasing depression storage of the subcatchment to 1".

BMP C-8: Due to lack of information, this infiltration BMP was assumed to treat the first 1" of rainfall based on stormwater regulations. This was done in the model by increasing depression storage of the subcatchment to 1".

BMP C-9: Due to lack of information, this infiltration BMP was assumed to treat the first 1" of rainfall based on stormwater regulations. This was done in the model by increasing depression storage of the subcatchment to 1".

BMP C-10: Due to lack of information, this infiltration BMP was assumed to treat the first 1" of rainfall based on stormwater regulations. This was done in the model by increasing depression storage of the subcatchment to 1".

BMP C-11: Due to lack of information, this infiltration BMP was assumed to treat the first 1" of rainfall based on stormwater regulations. This was done in the model by increasing depression storage of the subcatchment to 1".

BMP C-12: Due to lack of information, this dry pond was assumed to treat the first 1" of rainfall based on stormwater regulations. This was done in the model by increasing depression storage of the subcatchment to 1".

BMP C-13: Due to lack of information, this sand filter was assumed to treat the first 1" of rainfall based on stormwater regulations. This was done in the model by increasing depression storage of the subcatchment to 1".

BMP C-14: Due to lack of information, this infiltration BMP was assumed to treat the first 1" of rainfall based on stormwater regulations. This was done in the model by increasing depression storage of the subcatchment to 1".



3 Hydraulics

LimnoTech surveyed cross sections along Church Creek for the HEC-RAS model in September 2017. The focus of the survey was on defining the main channel flow path and the immediate floodplain. The elevation for the extended floodplains and overbank areas were taken from the LiDAR derived surface that was received from the County (Anne Arundel County 2017). Versar also has stream cross sections recorded in 2017 for a geomorphic study, and these cross sections were also used for the HEC-RAS model. In order to import the cross sections into HEC-RAS, cross sections were first taken from the LiDAR surface and then compared to the surveyed cross sections from LimnoTech and Versar. The cross sections were then adjusted to reflect the surveyed portion of the channel which helped to define the low flow channel more precisely. Generally, there was little difference between the LiDAR cross sections and the surveyed cross sections. In areas where the low flow channel was not picked up by the LiDAR, small modifications were made in HEC-RAS to reflect the low flow channel. The Solomons Island Road stream crossings was surveyed as part of the KCI study and these elevations and details were used directly in the HEC-RAS model. The final modeled cross sections are shown in Figure 3.

Manning’s n values which describe the flow roughness of the stream were assigned based on field notes and photographs of the site. For the main channel, an “n” value of 0.045, which corresponds to a main channel with vegetation, some pools, and stones/obstructions (Chow 1959), was used. Overbank “n” values of 0.08 - which corresponds to floodplains with light brush and trees - were used (Chow 1959).

A steady flow file was created to define the flows and model boundary condition. The 1, 2, 10, and 100-year discharges from the SWMM model were used to develop this steady flow file. The downstream slope of the channel (0.01 ft/ft), was entered into the model as the downstream “normal depth” boundary condition. The “subcritical” flow regime was selected because this is a low gradient stream.

The discharges from SWMM that were modeled in HEC-RAS are shown in Table 1.

River Station	1-year	2-year	10-year	100-year
2442	183	247	371	521
1664	280	374	558	781
942	539	723	1247	1833
654	548	734	1263	1856

The modeled BMPs provide volume and peak flow reduction, but are much less effective in doing so for larger storms. During small storms up to an inch, the BMPs are not at capacity, meaning that they can provide mitigation throughout the storm. Once the BMPs fill, however, they provide little to no flow mitigation and the outflow is equal to the inflow. Additionally, these large 10- and 100-year design storms are so intense that the BMPs fill in minutes, reducing their overall effectiveness to mitigate the impacts of these larger events.



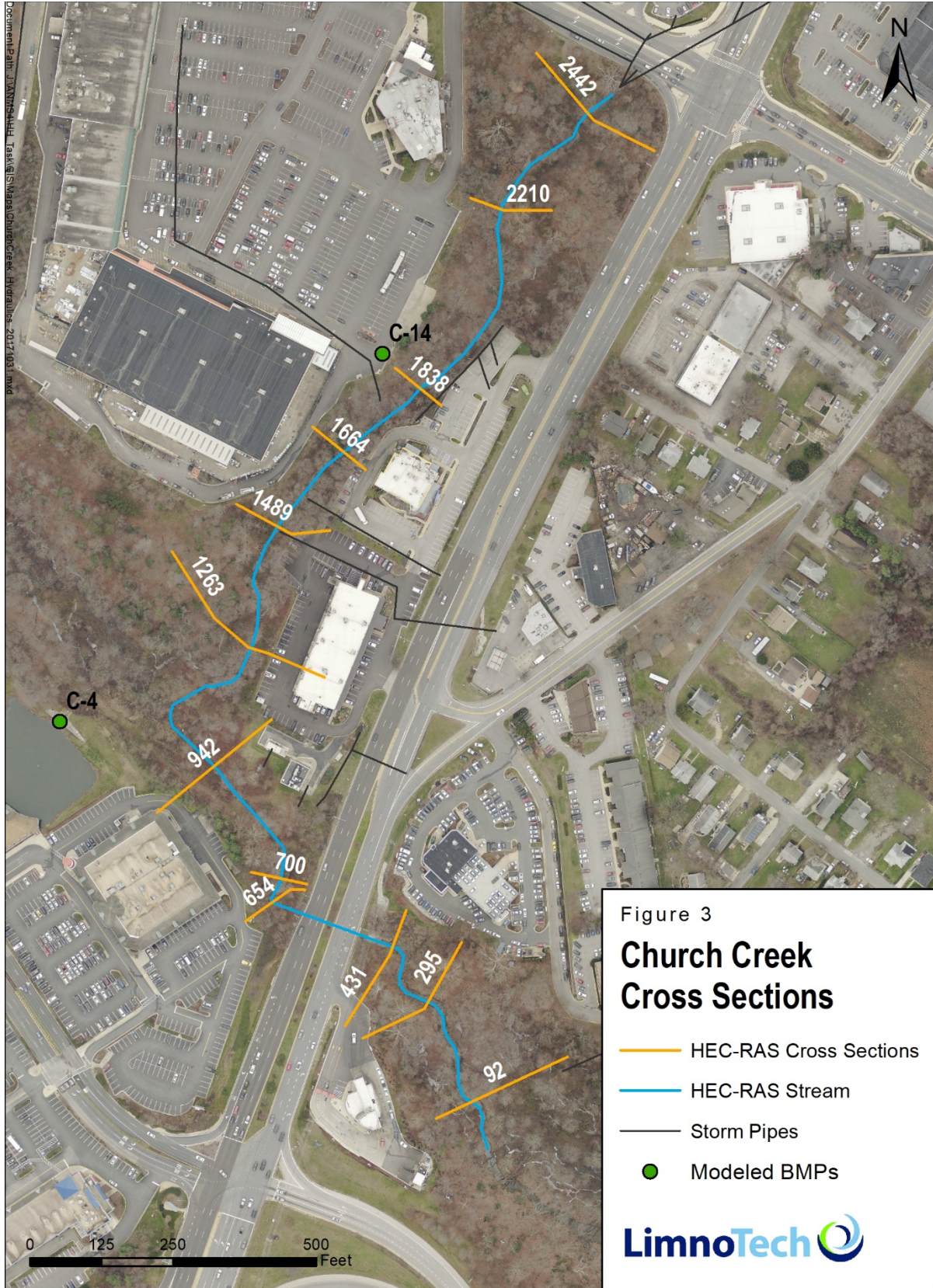


Figure 3. Church Creek Modeled Cross Sections



4 Results

To assess a stream's stability, the velocity and shear stress are both used to evaluate the erosive forces within the stream channel. The maximum permissible velocity of a stable silty loam stream channel is 3.0 ft/s (ASCE 1926). The critical bed shear stress in the channel should be less than 0.03 lb/ft² (USGS 2008) for similar soil conditions. It is important to note that all results presented below are values for the peak of the hydrograph and do not occur throughout the event. Therefore, if the velocity or shear stress is exceeded it does not necessarily mean the stream is not in equilibrium. If normal sediment transport conditions exist, the eroded areas will be replenished from upstream areas and the stream will remain stable.

1 year Recurrence Interval Event

The 1-year storm does not overtop Solomons Island Road. The channel velocities range from 7-9 ft/s at the upstream end of the reach where the slope is steepest to less than 3 ft/s at the downstream reach where the channel slope is more gradual. The shear stress in the channel varies throughout the reach but is highest where the velocity is the greatest. See Table 2 for hydraulic model results.

River Station	Maximum W.S. Elevation	Average Peak Channel Velocity	Maximum Shear Stress, Channel
	(ft)	(ft/s)	(lb/sq/ft)
2442	33.96	7.96	2.80
2210	30.37	3.74	0.58
1838	28.82	4.69	0.84
1664	26.72	9.07	3.12
1489	23.48	9.31	3.50
1263	21.77	1.59	0.09
942	21.73	1.39	0.06
700	21.59	3.13	0.28
654	21.36	4.11	0.57
595	Solomons Island Road		
431	14.10	9.23	3.30
295	12.23	8.44	2.80
92	10.28	5.53	1.35

2 year Recurrence Interval Event

The 2-year storm also does not overtop the crossing although flow is being restricted by the culvert. The velocity in the channel has a similar range to the 1-year storm with a slight increase. The water surface elevations do increase as expected for the 2-year recurrence interval.



Table 3. HEC-RAS Results for 2-Year Recurrence Interval			
River Station	W.S. Elevation	Average Channel Velocity	Shear Stress, Channel
	(ft)	(ft/s)	(lb/sq/ft)
2442	34.35	8.80	3.24
2210	30.85	3.70	0.54
1838	29.52	4.92	0.87
1664	27.01	10.90	4.39
1489	26.28	5.42	0.95
1263	26.51	0.66	0.01
942	26.50	1.04	0.03
700	26.45	2.10	0.11
654	26.39	2.61	0.20
595	Solomons Island Road		
431	14.78	9.97	3.65
295	12.53	10.15	3.93
92	10.53	5.94	1.51

10 year Recurrence Interval Event

For the 10-year storm event, the Solomons Island culvert is at capacity and the road is overtopped. The velocities and shear stresses in the channel are much lower in this scenario as flow is backed up behind the crossings, creating slow moving riverine conditions.

Table 4. HEC-RAS Results for 10-Year Recurrence Interval			
River Station	W.S. Elevation	Average Channel Velocity	Shear Stress, Channel
	(ft)	(ft/s)	(lb/sq/ft)
2442	35.10	9.64	3.58
2210	34.31	1.71	0.09
1838	34.16	2.54	0.18
1664	34.02	3.85	0.39
1489	34.01	2.48	0.15
1263	34.05	0.35	0.00
942	34.04	0.83	0.01
700	34.02	1.81	0.07
654	34.00	1.91	0.09
595	Solomons Island Road		



Table 4. HEC-RAS Results for 10-Year Recurrence Interval			
River Station	W.S. Elevation	Average Channel Velocity	Shear Stress, Channel
	(ft)	(ft/s)	(lb/sq/ft)
431	16.18	11.64	4.55
295	14.02	9.62	3.13
92	11.09	6.82	1.85

100 year Recurrence Interval Event

During the 100-year event, the culvert is again at capacity and the road is overtopped. Velocities are much slower in the channel because the water is backed up but erosion may occur just downstream of the crossing.

Table 5. HEC-RAS Results for 100-Year Recurrence Interval			
River Station	W.S. Elevation	Average Channel Velocity	Shear Stress, Channel
	(ft)	(ft/s)	(lb/sq/ft)
2442	35.86	10.14	3.69
2210	35.29	1.97	0.11
1838	35.11	3.07	0.25
1664	34.90	4.80	0.58
1489	34.90	3.04	0.22
1263	34.95	0.46	0.00
942	34.94	1.13	0.03
700	34.90	2.52	0.13
654	34.87	2.56	0.16
595	Solomons Island Road		
431	17.7	11.68	4.25
295	14.69	10.78	3.77
92	11.61	7.58	2.17



5 Discussion

The modeled results do show velocity and shear stress thresholds being exceeded in some locations. From our field observations, there was evidence of bank erosion and channel instability. The Parole Tributary of Church Creek is a relatively steep gradient stream (>1% slope) in a confined stream valley with little floodplain area. The contributing drainage area is highly impervious and runoff is likely flashy and immediate. Because of this hydrology and the lack of floodplain area, the stream energy remains confined in the channel and is causing erosion and instability. Additionally, yearly geomorphic monitoring from 2010 to 2017 has been performed in Parole Tributary and Church Creek and results from this monitoring corroborate the findings presented in this report. The monitoring found the conditions in the area have not improved over the years and remain in a degraded and impaired condition. Although the stream channel has been stabilized in some areas from restoration activities, the effects have yet to be broadly realized over the length of the stream.



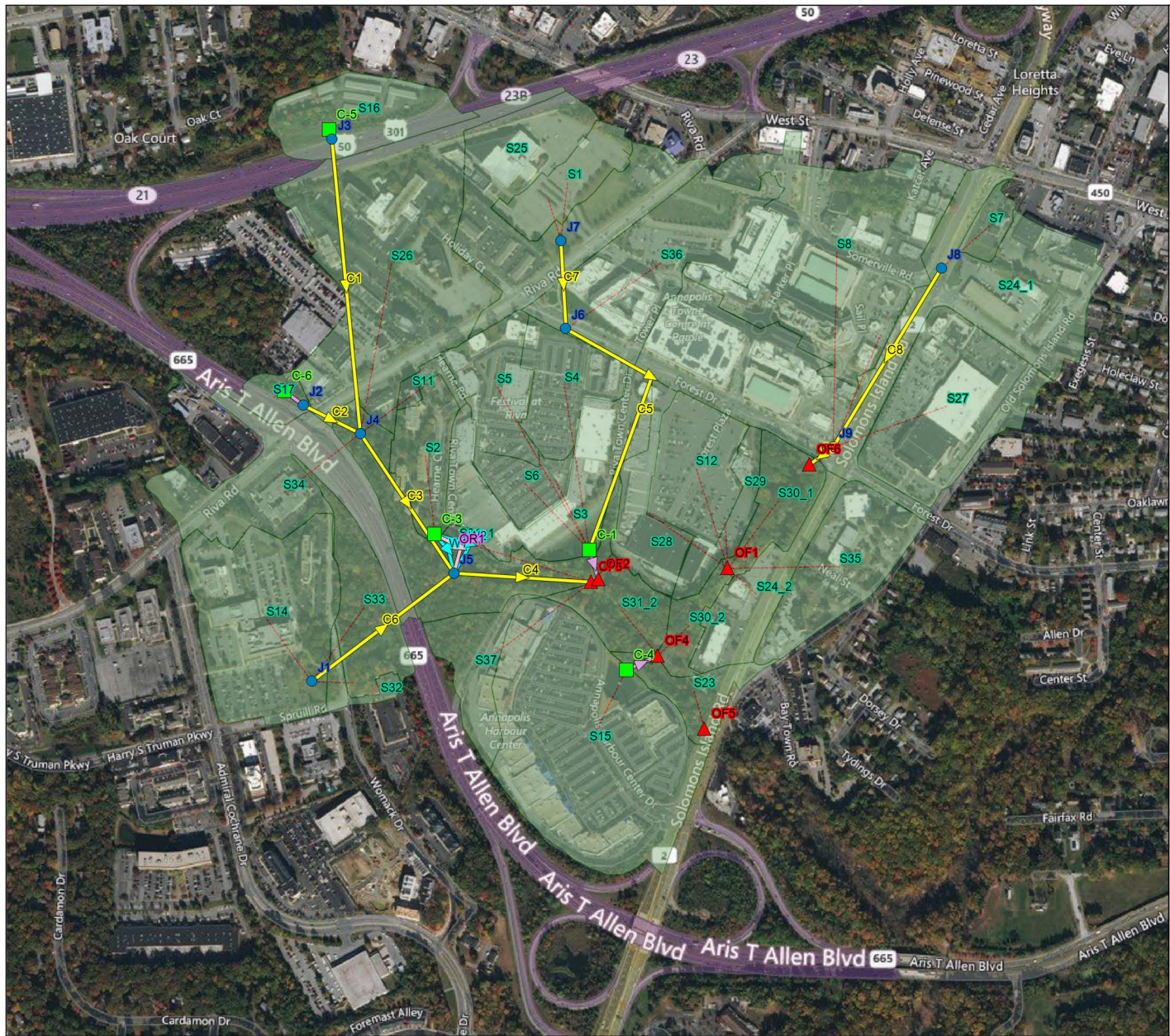
6 References

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- KCI Technologies, Inc. 2010. *Parole Tributary to Church Creek Hydrology and Hydraulic Analysis*. February 2010.
- McCrone. 2002. *West County Library Stormwater Management Report*. July 2002.
- USDA. 2016. Soil Survey Map of Anne Arundel County, SSURGO. Accessed through <https://websoilsurvey.sc.egov.usda.gov/App/HomePage.htm>.
- USGS. 2008. Scientific Investigations Report 2008-5093. Table 7.




Appendix A. SWMM Model Files





Legend

- Junctions
- ▲ Outfalls
- Storages
- Conduits
- Orifices
- Weirs
- Outlets
- Subcatchments



1000ft

[TITLE]

[OPTIONS]

```
;;Options          Value
;;-----
FLOW_UNITS        CFS
INFILTRATION      HORTON
FLOW_ROUTING      DYNWAVE
START_DATE        01/01/1989
START_TIME        00:00:00
REPORT_START_DATE 01/01/1989
REPORT_START_TIME 00:00:00
END_DATE          01/02/1989
END_TIME          00:00:00
SWEEP_START       01/01
SWEEP_END         12/31
DRY_DAYS          0
REPORT_STEP       00:01:00
WET_STEP          00:05:00
DRY_STEP          00:05:00
ROUTING_STEP      5
ALLOW_PONDING     NO
INERTIAL_DAMPING  PARTIAL
VARIABLE_STEP     0.75
LENGTHENING_STEP 0
MIN_SURFAREA     0
NORMAL_FLOW_LIMITED BOTH
SKIP_STEADY_STATE NO
FORCE_MAIN_EQUATION H-W
LINK_OFFSETS      DEPTH
MIN_SLOPE         0
MAX_TRIALS        8
HEAD_TOLERANCE    0.005
SYS_FLOW_TOL      5
LAT_FLOW_TOL      5
MINIMUM_STEP      0.5
THREADS           4
```

[EVAPORATION]

```
;;Type            Parameters
;;-----
CONSTANT          0.0
DRY_ONLY          NO
```

[RAINGAGES]

```
;;
;;Name            Rain    Time    Snow    Data
;;Type            Type    Intrvl  Catch   Source
;;-----
100yr_24hr       VOLUME 0:05   1.0    TIMESERIES 100yr_24hr
10yr_24hr        VOLUME 0:05   1.0    TIMESERIES 10yr_24hr
1yr_24hr         VOLUME 0:05   1.0    TIMESERIES 1yr_24hr
2yr_24hr         VOLUME 0:05   1.0    TIMESERIES 2yr_24hr
```

[SUBCATCHMENTS]

```
;;
;;Name            Raingage      Outlet          Total    Pcnt.    Width    Pcnt.    Curb    Snow
;;-----          -----          -----          Area    Imperv   -----    Slope   Length  Pack
;Modified Urban Stormwater BMP Review
S1  1yr_24hr      J7              4.3515  77.717  250      4.45    0
;Modified Urban Stormwater BMP Review
S11 1yr_24hr      J4              0.948   75.474  100      6.523   0
;Modified Urban Stormwater BMP Review
S12 1yr_24hr      OF1             6.7632  94.521  370      4.762   0
;Modified Urban Stormwater BMP Review
S14 1yr_24hr      J1              16.5114 65.022  700      6.424   0
;Modified KCI Drainage Area
S15 1yr_24hr      C-4             26.0041 85.496  120      5.233   0
;Modified KCI Drainage Area
S16 1yr_24hr      C-5             7.6515  62.032  560      10.863  0
;Modified KCI Drainage Area
S17 1yr_24hr      C-6             1.8694  43.848  250      17.354  0
;Modified Urban Stormwater BMP Review
S2  1yr_24hr      C-3             4.7466  46.975  250      10.435  0
;Modified KCI Drainage Area
S23 1yr_24hr      OF5             3.2955  27.963  210      13.355  0
;Modified KCI Drainage Area
S24_1 1yr_24hr     J9              30.3058 86.25   660      5.4     0
;Modified KCI Drainage Area
S24_2 1yr_24hr     OF1             5.5911  89.596  190      5.4     0
;Modified KCI Drainage Area
S25 1yr_24hr      J7              16.1406 69.63   470      7.912   0
;Modified KCI Drainage Area
S26 1yr_24hr      J4              26.0991 74.973  490      6.245   0
;Modified KCI Drainage Area
S27 1yr_24hr      J9              13.3416 85.032  530      5.731   0
;Modified KCI Drainage Area
S28 1yr_24hr      OF1             4.6769  98.206  170      3.378   0
;Modified KCI Drainage Area
S29 1yr_24hr      OF1             3.0003  84.624  190      5.82    0
;Modified Urban Stormwater BMP Review
S3  1yr_24hr      C-1             10.7295 82.724  250      7.108   0
;Modified KCI Drainage Area
S30_1 1yr_24hr     OF1             4.3801  0.62    240      26.16   0
;Modified KCI Drainage Area
```

S30_2	1yr_24hr	OF4	2.1699	0.997	150	26.16	0
;Modified KCI Drainage Area							
S31_1	1yr_24hr	OF3	10.9772	5.086	260	18.698	0
;Modified KCI Drainage Area							
S31_2	1yr_24hr	OF4	4.2986	1.135	250	18.698	0
;Modified KCI Drainage Area							
S32	1yr_24hr	J1	1.3294	72.484	490	5.268	0
;Modified KCI Drainage Area							
S33	1yr_24hr	J1	6.182	0.014	200	12.915	0
;Modified KCI Drainage Area							
S34	1yr_24hr	J4	12.2525	72.325	540	9.501	0
;Modified KCI Drainage Area							
S35	1yr_24hr	OF1	8.7548	70.645	320	7.796	0
;Modified KCI Drainage Area							
S36	1yr_24hr	J6	14.2682	67.63	440	5.444	0
;Modified KCI Drainage Area							
S37	1yr_24hr	OF3	4.7395	44.47	200	11.627	0
;Modified Urban Stormwater BMP Review							
S4	1yr_24hr	C-1	5.793	84.213	350	5.301	0
;Modified Urban Stormwater BMP Review							
S5	1yr_24hr	C-1	5.6003	82.905	330	4.545	0
;Modified Urban Stormwater BMP Review							
S6	1yr_24hr	C-1	3.0637	95.158	270	4.332	0
;Modified Urban Stormwater BMP Review							
S7	1yr_24hr	J8	1.5525	51.816	180	6.898	0
;Modified Urban Stormwater BMP Review							
S8	1yr_24hr	J9	22.6868	84.75	650	3.845	0

[SUBAREAS]

;;Subcatchment	N-Imperv	N-Perov	S-Imperv	S-Perov	PctZero	RouteTo	PctRouted
S1	0.01	0.1	1	1	0	OUTLET	
S11	0.01	0.1	1	1	0	OUTLET	
S12	0.01	0.1	1	1	0	OUTLET	
S14	0.01	0.1	1	1	0	OUTLET	
S15	0.01	0.1	0.05	0.05	25	OUTLET	
S16	0.01	0.1	0.05	0.05	25	OUTLET	
S17	0.01	0.1	0.05	0.05	25	OUTLET	
S2	0.01	0.1	0.05	0.05	25	OUTLET	
S23	0.01	0.1	0.05	0.05	25	OUTLET	
S24_1	0.01	0.1	0.05	0.05	25	OUTLET	
S24_2	0.01	0.1	0.05	0.05	25	OUTLET	
S25	0.01	0.1	0.05	0.05	25	OUTLET	
S26	0.01	0.1	0.05	0.05	25	OUTLET	
S27	0.01	0.1	0.05	0.05	25	OUTLET	
S28	0.01	0.1	0.05	0.05	25	OUTLET	
S29	0.01	0.1	0.05	0.05	25	OUTLET	
S3	0.01	0.1	0.05	0.05	25	OUTLET	
S30_1	0.01	0.1	0.05	0.05	25	OUTLET	
S30_2	0.01	0.1	0.05	0.05	25	OUTLET	
S31_1	0.01	0.1	0.05	0.05	25	OUTLET	
S31_2	0.01	0.1	0.05	0.05	25	OUTLET	
S32	0.01	0.1	0.05	0.05	25	OUTLET	
S33	0.01	0.1	0.05	0.05	25	OUTLET	
S34	0.01	0.1	0.05	0.05	25	OUTLET	
S35	0.01	0.1	0.05	0.05	25	OUTLET	
S36	0.01	0.1	0.05	0.05	25	OUTLET	
S37	0.01	0.1	0.05	0.05	25	OUTLET	
S4	0.01	0.1	1	1	0	OUTLET	
S5	0.01	0.1	1	1	0	OUTLET	
S6	0.01	0.1	1	1	0	OUTLET	
S7	0.01	0.1	1	1	0	OUTLET	
S8	0.01	0.1	1	1	0	OUTLET	

[INFILTRATION]

;;Subcatchment	MaxRate	MinRate	Decay	DryTime	MaxInfil
S1	1.5	0.112	5.257	3.044	0
S11	0.752	0.05	6.012	2.065	0
S12	0.752	0.05	6.012	2.065	0
S14	1.532	0.115	5.225	3.086	0
S15	0.839	0.056	5.924	2.085	0
S16	1.684	0.112	5.069	2.275	0
S17	2.11	0.141	4.637	2.371	0
S2	0.795	0.054	5.969	2.121	0
S23	1.35	0.09	5.407	2.2	0
S24_1	0.918	0.064	5.845	2.282	0
S24_2	2.035	0.137	4.714	2.414	0
S25	1.416	0.104	5.342	2.867	0
S26	0.911	0.061	5.852	2.101	0
S27	1.038	0.074	5.724	2.439	0
S28	0.755	0.05	6.009	2.069	0
S29	1.535	0.115	5.222	3.089	0
S3	0.76	0.051	6.004	2.075	0
S30_1	3.549	0.283	3.19	5.685	0
S30_2	2.121	0.161	4.63	3.642	0
S31_1	2.205	0.167	4.545	3.671	0
S31_2	1.962	0.151	4.791	3.649	0
S32	3.304	0.263	3.437	5.406	0
S33	3.426	0.268	3.313	5.219	0
S34	1.526	0.103	5.229	2.318	0
S35	1.712	0.123	5.042	2.834	0
S36	1.112	0.08	5.649	2.536	0
S37	1.646	0.123	5.11	3.164	0
S4	0.752	0.05	6.012	2.065	0

S5	0.752	0.05	6.012	2.065	0
S6	0.752	0.05	6.012	2.065	0
S7	0.752	0.05	6.012	2.065	0
S8	0.753	0.05	6.011	2.066	0

[JUNCTIONS]

;;Name	Invert Elev.	Max. Depth	Init. Depth	Surcharge Depth	Ponded Area
J1	55.767	0	0	0	0
;lowered due to low bottom elevation of us pond					
J2	60	0	0	0	0
J3	77	0	0	0	0
J4	50.73	0	0	0	0
J5	31.896	0	0	0	0
;Lowered invert elev. 10 ft from DEM under road					
J6	54.29	0	0	0	0
J7	70.046	0	0	0	0
J8	73.167	0	0	0	0
;lowered invert elevation 1 ft from DEM to make lower than road					
J9	44.596	0	0	0	0

[OUTFALLS]

;;Name	Invert Elev.	Outfall Type	Stage/Table Time Series	Tide Gate Route To
OF1	32.8	FREE		NO
OF2	27.805	FREE		NO
OF3	27.805	FREE		NO
OF4	22.554	FREE		NO
OF5	27.139	FREE		NO
OF6	33.932	FREE		NO

[STORAGE]

;;Name	Invert Elev.	Max. Depth	Init. Depth	Storage Curve	Curve Params	Ponded Area	Evap. Frac.	Infiltration parameters
;PWED-Extended Detention Structure, Wet								
C-1	28.7	11.3	0	TABULAR	CC_1	0	0	
;XDPD-Detention Structure (Dry Pond) (plain old pond)								
C-3	41	7	0	TABULAR	CC_4	0	0	
C-4	25.9	8.1	0	TABULAR	CC_5	0	0	
C-5	77	5	0	TABULAR	CC_7	0	0	
C-6	61	13	0	TABULAR	CC_8	0	0	

[CONDUITS]

;;Name	Inlet Node	Outlet Node	Length	Manning N	Inlet Offset	Outlet Offset	Init. Flow	Max. Flow
C1	J3	J4	1576.94	0.01	0	0	0	0
C2	J2	J4	339.06	0.01	0	0	0	0
C3	J4	J5	896.83	0.01	0	0	0	0
C4	J5	OF3	734.77	0.01	0	0	0	0
C5	J6	C-1	1505.33	0.01	0	0	0	0
C6	J1	J5	954.89	0.01	0	0	0	0
C7	J7	J6	470.63	0.01	0	0	0	0
C8	J8	J9	1115.39	0.01	0	0	0	0
C9	J9	OF6	157.6	0.01	0	0	0	0

[ORIFICES]

;;Name	Inlet Node	Outlet Node	Orifice Type	Crest Height	Disch. Coeff.	Flap Open/Close Gate Time
OR1	C-3	J5	SIDE	3.58	0.65	NO 0

[WEIRS]

;;Name	Inlet Node	Outlet Node	Weir Type	Crest Height	Disch. Coeff.	Flap End Gate Con.	End Coeff.	Surcharge
;emergency spillway								
W1	C-3	J5	TRAPEZOIDAL	5	3.33	NO 0	0	YES
W2	C-3	J5	TRANSVERSE	7	3.33	NO 0	0	YES

[OUTLETS]

;;Name	Inlet Node	Outlet Node	Outflow Height	Outlet Type	Qcoeff/QTable	Qexpon	Flap Gate
OL10	C-1	OF2	0	TABULAR/DEPTH	CCO_1		NO
OL7	C-6	J2	0	TABULAR/DEPTH	CCO_8		NO
OL8	C-5	J3	0	TABULAR/DEPTH	CCO_7		NO
OL9	C-4	OF4	0	TABULAR/DEPTH	CCO_5		NO

[XSECTIONS]

;;Link	Shape	Geom1	Geom2	Geom3	Geom4	Barrels
C1	CIRCULAR	1.25	0	0	0	1
C2	CIRCULAR	1	0	0	0	1
C3	TRAPEZOIDAL	20	3	3	3	1
C4	TRAPEZOIDAL	20	3	3	3	1
C5	CIRCULAR	4.5	0	0	0	1
C6	CIRCULAR	3.5	0	0	0	1
C7	CIRCULAR	3.5	0	0	0	1

C8	CIRCULAR	1.25	0	0	0	1
C9	CIRCULAR	4.5	0	0	0	1
OR1	CIRCULAR	1.25	0	0	0	
W1	TRAPEZOIDAL	2	8	2	2	
W2	RECT_OPEN	1	100	0	0	

```
[LOSSES]
;;Link      Inlet      Outlet      Average      Flap Gate      SeepageRate
;-----
```

```
[CURVES]
;;Name      Type      X-Value      Y-Value
;-----
```

CCO_1	Rating	0.21	2.76879E-63
CCO_1		0.851028744	1
CCO_1		1.03	1.093183965
CCO_1		1.235324122	3
CCO_1		1.856798439	4
CCO_1		2.618122561	5
CCO_1		7.099696384	10
CCO_1		7.260919529	15
CCO_1		7.377204582	20.001
CCO_1		7.477079002	25
CCO_1		7.567543881	30
CCO_1		7.651750486	35
CCO_1		7.731577682	40
CCO_1		7.80537322	45
CCO_1		7.876711998	50
CCO_1		8.94625	75
CCO_1		9.495	100
CCO_1		9.56125	125
CCO_1		9.6175	150
CCO_1		9.72	200
CCO_1		11.3	350

CCO_5	Rating	0.041464844	0.000273004
CCO_5		1.158774414	0.999989263
CCO_5		2.15712265	1.999876618
CCO_5		2.207419434	3.001682535
CCO_5		2.247702637	4.000324832
CCO_5		2.289499512	4.995344068
CCO_5		2.330328064	6.000024624
CCO_5		2.372392578	6.977235885
CCO_5		2.76	8
CCO_5		3.475	9
CCO_5		4.19	10
CCO_5		4.27	20
CCO_5		4.32	30
CCO_5		4.395	45
CCO_5		4.42	50
CCO_5		4.470005	60.001
CCO_5		4.520005	70.001
CCO_5		4.57	80
CCO_5		4.62	90
CCO_5		4.67	100

CCO_7	Rating	0.83	0
CCO_7		1.670976155	1
CCO_7		2.08	0.956687194
CCO_7		2.24652116	3
CCO_7		2.833259841	4
CCO_7		3.477281001	5
CCO_7		4.178584641	6
CCO_7		4.576679738	7
CCO_7		4.660210673	8
CCO_7		4.733672588	9
CCO_7		6.62375	15
CCO_7		7.032857143	20
CCO_7		7.061428571	25
CCO_7		7.09	30
CCO_7		7.123333333	40
CCO_7		7.15625	50
CCO_7		7.1875	60
CCO_7		7.215	70
CCO_7		7.24	80

CCO_8	Rating	3.002929688	0.000491579
CCO_8		3.470581055	1.00072178
CCO_8		3.745605469	1.995838155
CCO_8		4.128662109	2.998421671
CCO_8		4.618835449	4.00006616
CCO_8		5.247802734	4.999542593
CCO_8		6.016845703	5.999462751
CCO_8		6.542816162	6.999532618
CCO_8		6.595687866	7.999790647
CCO_8		6.636932373	9.002232529
CCO_8		6.672546387	9.995279702
CCO_8		6.704956055	10.98981355
CCO_8		6.735622559	12.00377529
CCO_8		6.763813477	12.99420333
CCO_8		6.791088867	14.00264031
CCO_8		6.818181152	15.00717449
CCO_8		6.960051618	19.97377548
CCO_8		7.091256278	24.99642553
CCO_8		11.26111111	40

CCO_8		15.25375	50
CC_1	Storage	0	0
CC_1		3.3	19115
CC_1		7.3	26817
CC_1		9.3	32520
CC_1		11.3	39480
CC_4	Storage	0	0
CC_4		1	6480
CC_4		5	10460
CC_4		7	12503
CC_5	Storage	0	0
CC_5		0.1	22674
CC_5		4.1	30230
CC_5		6.1	33260
CC_5		8.1	42240
CC_7	Storage	0	0
CC_7		1	16050
CC_7		3	24717
CC_7		5	77290
CC_8	Storage	0	0
CC_8		5	5025
CC_8		9	9560
CC_8		11	12368
CC_8		13	16360

[TIMESERIES]

;;Name	Date	Time	Value
;;-----	-----	-----	-----
100yr_24hr	1/1/1989	0:00	0.0000
100yr_24hr	1/1/1989	0:05	0.0108
100yr_24hr	1/1/1989	0:10	0.0108
100yr_24hr	1/1/1989	0:15	0.0108
100yr_24hr	1/1/1989	0:20	0.0108
100yr_24hr	1/1/1989	0:25	0.0108
100yr_24hr	1/1/1989	0:30	0.0108
100yr_24hr	1/1/1989	0:35	0.0108
100yr_24hr	1/1/1989	0:40	0.0108
100yr_24hr	1/1/1989	0:45	0.0108
100yr_24hr	1/1/1989	0:50	0.0108
100yr_24hr	1/1/1989	0:55	0.0108
100yr_24hr	1/1/1989	1:00	0.0108
100yr_24hr	1/1/1989	1:05	0.0108
100yr_24hr	1/1/1989	1:10	0.0108
100yr_24hr	1/1/1989	1:15	0.0108
100yr_24hr	1/1/1989	1:20	0.0108
100yr_24hr	1/1/1989	1:25	0.0108
100yr_24hr	1/1/1989	1:30	0.0108
100yr_24hr	1/1/1989	1:35	0.0108
100yr_24hr	1/1/1989	1:40	0.0108
100yr_24hr	1/1/1989	1:45	0.0108
100yr_24hr	1/1/1989	1:50	0.0108
100yr_24hr	1/1/1989	1:55	0.0108
100yr_24hr	1/1/1989	2:00	0.0108
100yr_24hr	1/1/1989	2:05	0.0108
100yr_24hr	1/1/1989	2:10	0.0108
100yr_24hr	1/1/1989	2:15	0.0108
100yr_24hr	1/1/1989	2:20	0.0108
100yr_24hr	1/1/1989	2:25	0.0108
100yr_24hr	1/1/1989	2:30	0.0108
100yr_24hr	1/1/1989	2:35	0.0108
100yr_24hr	1/1/1989	2:40	0.0108
100yr_24hr	1/1/1989	2:45	0.0108
100yr_24hr	1/1/1989	2:50	0.0108
100yr_24hr	1/1/1989	2:55	0.0108
100yr_24hr	1/1/1989	3:00	0.0108
100yr_24hr	1/1/1989	3:05	0.0108
100yr_24hr	1/1/1989	3:10	0.0108
100yr_24hr	1/1/1989	3:15	0.0108
100yr_24hr	1/1/1989	3:20	0.0108
100yr_24hr	1/1/1989	3:25	0.0108
100yr_24hr	1/1/1989	3:30	0.0108
100yr_24hr	1/1/1989	3:35	0.0108
100yr_24hr	1/1/1989	3:40	0.0108
100yr_24hr	1/1/1989	3:45	0.0108
100yr_24hr	1/1/1989	3:50	0.0108
100yr_24hr	1/1/1989	3:55	0.0108
100yr_24hr	1/1/1989	4:00	0.0108
100yr_24hr	1/1/1989	4:05	0.0108
100yr_24hr	1/1/1989	4:10	0.0108
100yr_24hr	1/1/1989	4:15	0.0108
100yr_24hr	1/1/1989	4:20	0.0108
100yr_24hr	1/1/1989	4:25	0.0108
100yr_24hr	1/1/1989	4:30	0.0108
100yr_24hr	1/1/1989	4:35	0.0108
100yr_24hr	1/1/1989	4:40	0.0108
100yr_24hr	1/1/1989	4:45	0.0108
100yr_24hr	1/1/1989	4:50	0.0108
100yr_24hr	1/1/1989	4:55	0.0108
100yr_24hr	1/1/1989	5:00	0.0108
100yr_24hr	1/1/1989	5:05	0.0108

10yr_24hr	1/1/1989	4:50	0.0050
10yr_24hr	1/1/1989	4:55	0.0050
10yr_24hr	1/1/1989	5:00	0.0050
10yr_24hr	1/1/1989	5:05	0.0050
10yr_24hr	1/1/1989	5:10	0.0050
10yr_24hr	1/1/1989	5:15	0.0050
10yr_24hr	1/1/1989	5:20	0.0050
10yr_24hr	1/1/1989	5:25	0.0050
10yr_24hr	1/1/1989	5:30	0.0050
10yr_24hr	1/1/1989	5:35	0.0050
10yr_24hr	1/1/1989	5:40	0.0050
10yr_24hr	1/1/1989	5:45	0.0050
10yr_24hr	1/1/1989	5:50	0.0050
10yr_24hr	1/1/1989	5:55	0.0050
10yr_24hr	1/1/1989	6:00	0.0050
10yr_24hr	1/1/1989	6:05	0.0100
10yr_24hr	1/1/1989	6:10	0.0100
10yr_24hr	1/1/1989	6:15	0.0100
10yr_24hr	1/1/1989	6:20	0.0100
10yr_24hr	1/1/1989	6:25	0.0100
10yr_24hr	1/1/1989	6:30	0.0100
10yr_24hr	1/1/1989	6:35	0.0100
10yr_24hr	1/1/1989	6:40	0.0100
10yr_24hr	1/1/1989	6:45	0.0100
10yr_24hr	1/1/1989	6:50	0.0100
10yr_24hr	1/1/1989	6:55	0.0100
10yr_24hr	1/1/1989	7:00	0.0100
10yr_24hr	1/1/1989	7:05	0.0100
10yr_24hr	1/1/1989	7:10	0.0100
10yr_24hr	1/1/1989	7:15	0.0100
10yr_24hr	1/1/1989	7:20	0.0100
10yr_24hr	1/1/1989	7:25	0.0100
10yr_24hr	1/1/1989	7:30	0.0100
10yr_24hr	1/1/1989	7:35	0.0100
10yr_24hr	1/1/1989	7:40	0.0100
10yr_24hr	1/1/1989	7:45	0.0100
10yr_24hr	1/1/1989	7:50	0.0100
10yr_24hr	1/1/1989	7:55	0.0100
10yr_24hr	1/1/1989	8:00	0.0100
10yr_24hr	1/1/1989	8:05	0.0100
10yr_24hr	1/1/1989	8:10	0.0100
10yr_24hr	1/1/1989	8:15	0.0100
10yr_24hr	1/1/1989	8:20	0.0100
10yr_24hr	1/1/1989	8:25	0.0100
10yr_24hr	1/1/1989	8:30	0.0100
10yr_24hr	1/1/1989	8:35	0.0100
10yr_24hr	1/1/1989	8:40	0.0100
10yr_24hr	1/1/1989	8:45	0.0100
10yr_24hr	1/1/1989	8:50	0.0100
10yr_24hr	1/1/1989	8:55	0.0100
10yr_24hr	1/1/1989	9:00	0.0100
10yr_24hr	1/1/1989	9:05	0.0175
10yr_24hr	1/1/1989	9:10	0.0175
10yr_24hr	1/1/1989	9:15	0.0175
10yr_24hr	1/1/1989	9:20	0.0175
10yr_24hr	1/1/1989	9:25	0.0175
10yr_24hr	1/1/1989	9:30	0.0175
10yr_24hr	1/1/1989	9:35	0.0175
10yr_24hr	1/1/1989	9:40	0.0175
10yr_24hr	1/1/1989	9:45	0.0175
10yr_24hr	1/1/1989	9:50	0.0175
10yr_24hr	1/1/1989	9:55	0.0175
10yr_24hr	1/1/1989	10:00	0.0175
10yr_24hr	1/1/1989	10:05	0.0175
10yr_24hr	1/1/1989	10:10	0.0175
10yr_24hr	1/1/1989	10:15	0.0175
10yr_24hr	1/1/1989	10:20	0.0175
10yr_24hr	1/1/1989	10:25	0.0175
10yr_24hr	1/1/1989	10:30	0.0175
10yr_24hr	1/1/1989	10:35	0.0175
10yr_24hr	1/1/1989	10:40	0.0175
10yr_24hr	1/1/1989	10:45	0.0175
10yr_24hr	1/1/1989	10:50	0.0175
10yr_24hr	1/1/1989	10:55	0.0175
10yr_24hr	1/1/1989	11:00	0.0175
10yr_24hr	1/1/1989	11:05	0.0317
10yr_24hr	1/1/1989	11:10	0.0317
10yr_24hr	1/1/1989	11:15	0.0317
10yr_24hr	1/1/1989	11:20	0.0317
10yr_24hr	1/1/1989	11:25	0.0317
10yr_24hr	1/1/1989	11:30	0.0317
10yr_24hr	1/1/1989	11:35	0.0825
10yr_24hr	1/1/1989	11:40	0.0825
10yr_24hr	1/1/1989	11:45	0.0825
10yr_24hr	1/1/1989	11:50	0.1700
10yr_24hr	1/1/1989	11:55	0.2367
10yr_24hr	1/1/1989	12:00	0.5617
10yr_24hr	1/1/1989	12:05	0.3367
10yr_24hr	1/1/1989	12:10	0.1700
10yr_24hr	1/1/1989	12:15	0.1700
10yr_24hr	1/1/1989	12:20	0.0825
10yr_24hr	1/1/1989	12:25	0.0825
10yr_24hr	1/1/1989	12:30	0.0825
10yr_24hr	1/1/1989	12:35	0.0317
10yr_24hr	1/1/1989	12:40	0.0317

1yr_24hr	1/1/1989	4:30	0.0025
1yr_24hr	1/1/1989	4:35	0.0025
1yr_24hr	1/1/1989	4:40	0.0025
1yr_24hr	1/1/1989	4:45	0.0025
1yr_24hr	1/1/1989	4:50	0.0025
1yr_24hr	1/1/1989	4:55	0.0025
1yr_24hr	1/1/1989	5:00	0.0025
1yr_24hr	1/1/1989	5:05	0.0025
1yr_24hr	1/1/1989	5:10	0.0025
1yr_24hr	1/1/1989	5:15	0.0025
1yr_24hr	1/1/1989	5:20	0.0025
1yr_24hr	1/1/1989	5:25	0.0025
1yr_24hr	1/1/1989	5:30	0.0025
1yr_24hr	1/1/1989	5:35	0.0025
1yr_24hr	1/1/1989	5:40	0.0025
1yr_24hr	1/1/1989	5:45	0.0025
1yr_24hr	1/1/1989	5:50	0.0025
1yr_24hr	1/1/1989	5:55	0.0025
1yr_24hr	1/1/1989	6:00	0.0025
1yr_24hr	1/1/1989	6:05	0.0058
1yr_24hr	1/1/1989	6:10	0.0058
1yr_24hr	1/1/1989	6:15	0.0058
1yr_24hr	1/1/1989	6:20	0.0058
1yr_24hr	1/1/1989	6:25	0.0058
1yr_24hr	1/1/1989	6:30	0.0058
1yr_24hr	1/1/1989	6:35	0.0058
1yr_24hr	1/1/1989	6:40	0.0058
1yr_24hr	1/1/1989	6:45	0.0058
1yr_24hr	1/1/1989	6:50	0.0058
1yr_24hr	1/1/1989	6:55	0.0058
1yr_24hr	1/1/1989	7:00	0.0058
1yr_24hr	1/1/1989	7:05	0.0058
1yr_24hr	1/1/1989	7:10	0.0058
1yr_24hr	1/1/1989	7:15	0.0058
1yr_24hr	1/1/1989	7:20	0.0058
1yr_24hr	1/1/1989	7:25	0.0058
1yr_24hr	1/1/1989	7:30	0.0058
1yr_24hr	1/1/1989	7:35	0.0058
1yr_24hr	1/1/1989	7:40	0.0058
1yr_24hr	1/1/1989	7:45	0.0058
1yr_24hr	1/1/1989	7:50	0.0058
1yr_24hr	1/1/1989	7:55	0.0058
1yr_24hr	1/1/1989	8:00	0.0058
1yr_24hr	1/1/1989	8:05	0.0058
1yr_24hr	1/1/1989	8:10	0.0058
1yr_24hr	1/1/1989	8:15	0.0058
1yr_24hr	1/1/1989	8:20	0.0058
1yr_24hr	1/1/1989	8:25	0.0058
1yr_24hr	1/1/1989	8:30	0.0058
1yr_24hr	1/1/1989	8:35	0.0058
1yr_24hr	1/1/1989	8:40	0.0058
1yr_24hr	1/1/1989	8:45	0.0058
1yr_24hr	1/1/1989	8:50	0.0058
1yr_24hr	1/1/1989	8:55	0.0058
1yr_24hr	1/1/1989	9:00	0.0058
1yr_24hr	1/1/1989	9:05	0.0067
1yr_24hr	1/1/1989	9:10	0.0067
1yr_24hr	1/1/1989	9:15	0.0067
1yr_24hr	1/1/1989	9:20	0.0067
1yr_24hr	1/1/1989	9:25	0.0067
1yr_24hr	1/1/1989	9:30	0.0067
1yr_24hr	1/1/1989	9:35	0.0100
1yr_24hr	1/1/1989	9:40	0.0100
1yr_24hr	1/1/1989	9:45	0.0100
1yr_24hr	1/1/1989	9:50	0.0100
1yr_24hr	1/1/1989	9:55	0.0100
1yr_24hr	1/1/1989	10:00	0.0100
1yr_24hr	1/1/1989	10:05	0.0100
1yr_24hr	1/1/1989	10:10	0.0100
1yr_24hr	1/1/1989	10:15	0.0100
1yr_24hr	1/1/1989	10:20	0.0100
1yr_24hr	1/1/1989	10:25	0.0100
1yr_24hr	1/1/1989	10:30	0.0100
1yr_24hr	1/1/1989	10:35	0.0100
1yr_24hr	1/1/1989	10:40	0.0100
1yr_24hr	1/1/1989	10:45	0.0100
1yr_24hr	1/1/1989	10:50	0.0100
1yr_24hr	1/1/1989	10:55	0.0100
1yr_24hr	1/1/1989	11:00	0.0100
1yr_24hr	1/1/1989	11:05	0.0183
1yr_24hr	1/1/1989	11:10	0.0183
1yr_24hr	1/1/1989	11:15	0.0183
1yr_24hr	1/1/1989	11:20	0.0183
1yr_24hr	1/1/1989	11:25	0.0183
1yr_24hr	1/1/1989	11:30	0.0183
1yr_24hr	1/1/1989	11:35	0.0392
1yr_24hr	1/1/1989	11:40	0.0392
1yr_24hr	1/1/1989	11:45	0.0392
1yr_24hr	1/1/1989	11:50	0.0875
1yr_24hr	1/1/1989	11:55	0.1392
1yr_24hr	1/1/1989	12:00	0.3517
1yr_24hr	1/1/1989	12:05	0.2117
1yr_24hr	1/1/1989	12:10	0.0875
1yr_24hr	1/1/1989	12:15	0.0875
1yr_24hr	1/1/1989	12:20	0.0392

2yr_24hr	1/1/1989	20:00	0.0025
2yr_24hr	1/1/1989	20:05	0.0025
2yr_24hr	1/1/1989	20:10	0.0025
2yr_24hr	1/1/1989	20:15	0.0025
2yr_24hr	1/1/1989	20:20	0.0025
2yr_24hr	1/1/1989	20:25	0.0025
2yr_24hr	1/1/1989	20:30	0.0025
2yr_24hr	1/1/1989	20:35	0.0025
2yr_24hr	1/1/1989	20:40	0.0025
2yr_24hr	1/1/1989	20:45	0.0025
2yr_24hr	1/1/1989	20:50	0.0025
2yr_24hr	1/1/1989	20:55	0.0025
2yr_24hr	1/1/1989	21:00	0.0025
2yr_24hr	1/1/1989	21:05	0.0025
2yr_24hr	1/1/1989	21:10	0.0025
2yr_24hr	1/1/1989	21:15	0.0025
2yr_24hr	1/1/1989	21:20	0.0025
2yr_24hr	1/1/1989	21:25	0.0025
2yr_24hr	1/1/1989	21:30	0.0025
2yr_24hr	1/1/1989	21:35	0.0025
2yr_24hr	1/1/1989	21:40	0.0025
2yr_24hr	1/1/1989	21:45	0.0025
2yr_24hr	1/1/1989	21:50	0.0025
2yr_24hr	1/1/1989	21:55	0.0025
2yr_24hr	1/1/1989	22:00	0.0025
2yr_24hr	1/1/1989	22:05	0.0025
2yr_24hr	1/1/1989	22:10	0.0025
2yr_24hr	1/1/1989	22:15	0.0025
2yr_24hr	1/1/1989	22:20	0.0025
2yr_24hr	1/1/1989	22:25	0.0025
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2yr_24hr	1/1/1989	22:35	0.0025
2yr_24hr	1/1/1989	22:40	0.0025
2yr_24hr	1/1/1989	22:45	0.0025
2yr_24hr	1/1/1989	22:50	0.0025
2yr_24hr	1/1/1989	22:55	0.0025
2yr_24hr	1/1/1989	23:00	0.0025
2yr_24hr	1/1/1989	23:05	0.0025
2yr_24hr	1/1/1989	23:10	0.0025
2yr_24hr	1/1/1989	23:15	0.0025
2yr_24hr	1/1/1989	23:20	0.0025
2yr_24hr	1/1/1989	23:25	0.0025
2yr_24hr	1/1/1989	23:30	0.0025
2yr_24hr	1/1/1989	23:35	0.0025
2yr_24hr	1/1/1989	23:40	0.0025
2yr_24hr	1/1/1989	23:45	0.0025
2yr_24hr	1/1/1989	23:50	0.0025
2yr_24hr	1/1/1989	23:55	0.0025

[REPORT]
INPUT YES
CONTROLS NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL

[TAGS]

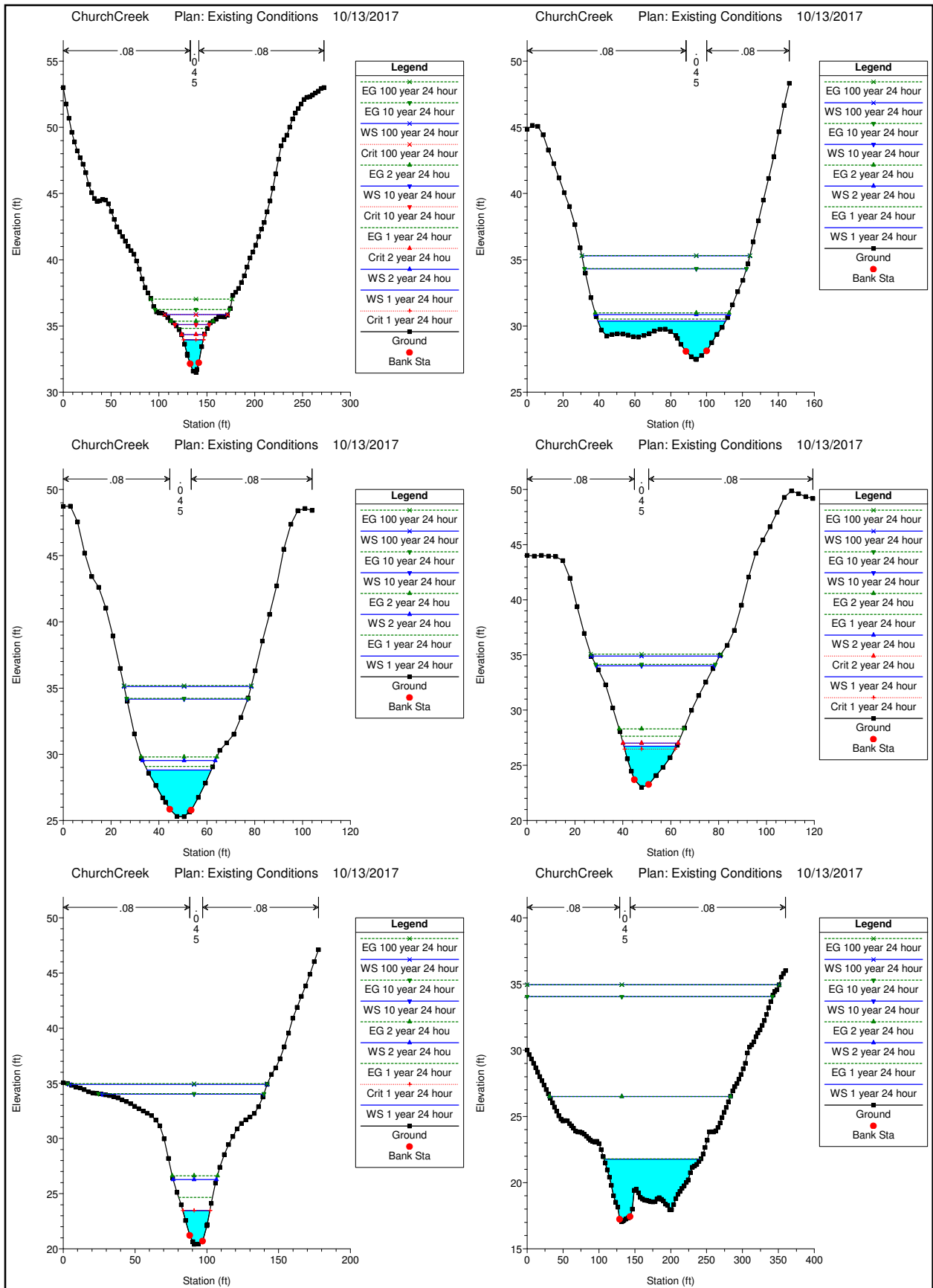
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UNITS Feet

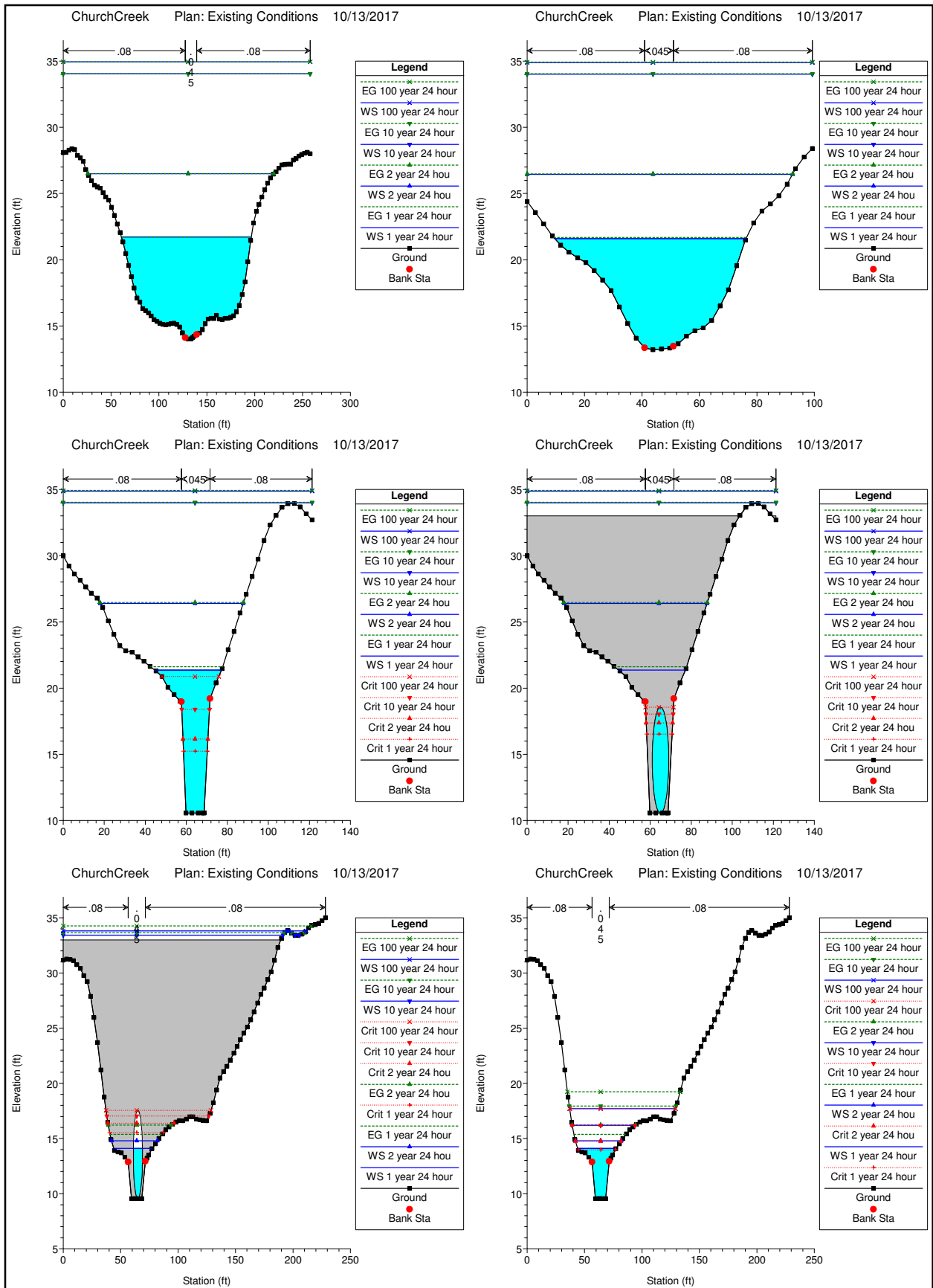
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; ;-----
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J2 1440552.217 478474.392
J3 1440696.956 479893.657
J4 1440856.524 478324.923
J5 1441361.86 477584.281
J6 1441953.504 478890.037
J7 1441923.895 479359.686
J8 1443960.932 479222.442
J9 1443385.705 478266.924
OF1 1442824.879 477617.825
OF2 1442132.936 477556.659
OF3 1442095.36 477541.59
OF4 1442453.174 477150.537
OF5 1442704.209 476763.4
OF6 1443259.46 478172.62
C-1 1442085.253 477711.628
C-3 1441256.812 477791.444
C-4 1442285.726 477073.524
C-5 1440681.707 479944.636
C-6 1440450.652 478550.611

[VERTICES]
; ;Link X-Coord Y-Coord
; ;-----
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OR1 1441395.76 477723.443
W1 1441332.391 477707.33
W2 1441448.555 477739.504

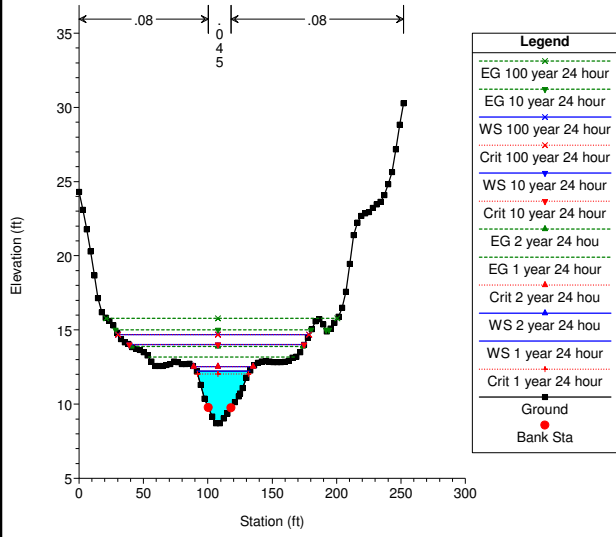
Appendix B. HEC-RAS Files



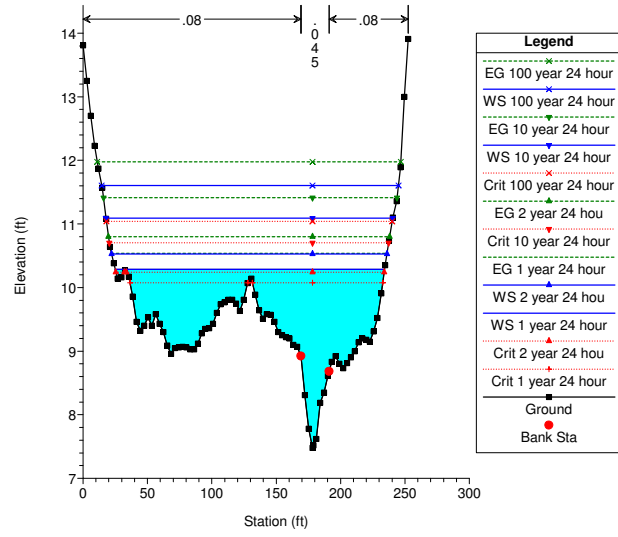




ChurchCreek Plan: Existing Conditions 10/13/2017



ChurchCreek Plan: Existing Conditions 10/13/2017



HEC-RAS Version 4.1.0 Jan 2010
U.S. Army Corps of Engineers
Hydrologic Engineering Center
609 Second Street
Davis, California

```
X  X XXXXXX  XXXX   XXXX   XX   XXXX
X  X X   X  X   X  X   X  X   X
X  X X   X     X  X   X  X   X
XXXXXXXX XXXX   X   XXX XXXX   XXXXXX   XXXX
X  X X   X     X  X   X  X   X
X  X X   X  X   X  X   X  X   X
X  X XXXXXX  XXXX   X  X  X  X  XXXXXX
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PROJECT DATA

Project Title: ChurchCreek
Project File : ChurchCreek.prj
Run Date and Time: 10/13/2017 10:26:16 AM

Project in English units

PLAN DATA

Plan Title: Existing Conditions
Plan File : j:\ANMS4\HH_Task\Hydraulics\HECRAS\model\CC\ChurchCreek.p01

Geometry Title: PSB GeoRAS
Geometry File : j:\ANMS4\HH_Task\Hydraulics\HECRAS\model\CC\ChurchCreek.g01

Flow Title : SWMM Model Flows
Flow File : j:\ANMS4\HH_Task\Hydraulics\HECRAS\model\CC\ChurchCreek.f01

Plan Summary Information:

Number of: Cross Sections = 12 Multiple Openings = 0
Culverts = 1 Inline Structures = 0
Bridges = 0 Lateral Structures = 0

Computational Information

Water surface calculation tolerance = 0.01
Critical depth calculation tolerance = 0.01
Maximum number of iterations = 20
Maximum difference tolerance = 0.3
Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary

Conveyance Calculation Method: At breaks in n values only
 Friction Slope Method: Average Conveyance
 Computational Flow Regime: Subcritical Flow

FLOW DATA

Flow Title: SWMM Model Flows
 Flow File : j:\ANMS4\HH_Task\Hydraulics\HECRAS\model\CC\ChurchCreek.f01

Flow Data (cfs)

River	Reach	RS	1 year 24 hour	2 year 24 hou	10 year 24 hour	100 year 24 hour
ChurchCrk	Reach1	2442	183	247	371	521
ChurchCrk	Reach1	1664	280	374	558	781
ChurchCrk	Reach1	942	539	723	1247	1833
ChurchCrk	Reach1	654	548	734	1263	1856

Boundary Conditions

River	Reach	Profile	Upstream	Downstream
ChurchCrk	Reach1	1 year 24 hour		Normal S = 0.01
ChurchCrk	Reach1	2 year 24 hou		Normal S = 0.01
ChurchCrk	Reach1	10 year 24 hour		Normal S = 0.01
ChurchCrk	Reach1	100 year 24 hour		Normal S = 0.01

GEOMETRY DATA

Geometry Title: PSB GeoRAS
 Geometry File : j:\ANMS4\HH_Task\Hydraulics\HECRAS\model\CC\ChurchCreek.g01

CROSS SECTION

RIVER: ChurchCrk
 REACH: Reach1 RS: 2442

INPUT

Description:

Station Elevation Data num= 95

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	53	2.95	51.77	5.9	50.69	8.85	49.62	11.8	48.9
14.74	48.22	17.69	47.7	20.64	47.21	23.59	46.57	26.54	45.69
29.49	45.07	32.44	44.63	35.39	44.41	38.34	44.45	41.28	44.55
44.23	44.51	47.18	44.21	50.13	43.64	53.08	43.05	56.03	42.47
58.98	42.12	61.93	41.76	64.88	41.39	67.83	41.02	70.77	40.7
73.72	40.43	76.67	39.91	79.62	39.3	82.57	38.56	85.52	37.9

88.47 37.5 91.42 37.08 94.36 36.47 97.31 36.06 100.26 35.99
 103.21 35.97 106.16 35.86 109.11 35.62 112.06 35.39 115.01 35.23
 117.96 35 120.93 34.7 123.89 34.34 126.86 33.63 129.53 32.85
 129.83 32.76 132.8 32.13 135.77 31.59 138.74 31.49 139.67 31.72
 141.7 32.21 144.67 33.44 147.64 34.38 150.61 34.82 153.58 35.23
 156.55 35.36 159.52 35.5 162.48 35.72 165.45 35.72 168.42 35.68
 171.39 35.82 174.36 36.33 177.33 37.29 180.3 37.55 183.26 37.84
 186.23 38.28 189.2 38.78 192.17 39.44 195.14 40.14 198.11 40.58
 201.08 41.11 204.04 41.73 207.01 42.32 209.98 42.81 212.95 43.62
 215.92 44.44 218.89 45.38 221.86 46.49 224.82 47.6 227.79 48.6
 230.76 49.07 233.73 49.41 236.7 50.03 239.67 50.63 242.63 51.08
 245.6 51.42 248.57 51.76 251.54 52.11 254.51 52.26 257.48 52.31
 260.45 52.44 263.41 52.59 266.38 52.74 269.35 52.94 272.32 53

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .08 132.8 .045 141.7 .08

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 132.8 141.7 225.53 232.08 237.13 .1 .3

CROSS SECTION

RIVER: ChurchCrk
 REACH: Reach1 RS: 2210

INPUT
 Description:

Station Elevation Data num= 53
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
 0 44.86 2.96 45.15 5.91 45.08 8.87 44.43 11.83 43.29
 14.79 42.26 17.74 41.17 20.7 40.06 23.66 39.01 26.62 37.65
 29.57 35.89 32.53 33.98 35.49 32.13 38.45 30.71 41.4 29.69
 44.36 29.25 47.32 29.35 50.28 29.41 53.23 29.38 56.19 29.31
 59.15 29.18 62.1 29.15 65.06 29.28 68.02 29.43 70.98 29.6
 73.93 29.75 76.89 29.77 79.85 29.59 82.81 29.28 83.72 29.07
 85.68 28.61 88.56 28.08 91.43 27.67 93.84 27.5 94.31 27.47
 97.18 27.76 100.06 28.11 102.93 28.74 105.81 29.37 108.68 29.89
 111.56 30.63 114.43 31.59 117.31 32.59 120.18 33.44 123.06 34.7
 125.93 36.34 128.81 37.94 131.68 39.51 134.55 41.14 137.43 42.8
 140.3 44.67 143.18 46.66 146.05 48.32

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .08 88.56 .045 100.06 .08

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 88.56 100.06 376.59 371.84 368.59 .1 .3

CROSS SECTION

RIVER: ChurchCrk

REACH: Reach1 RS: 1838

INPUT

Description:

Station Elevation Data num= 38

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	48.73	2.97	48.74	5.94	47.56	8.92	45.2	11.89	43.42
14.86	42.62	17.83	41.05	20.8	38.93	23.77	36.48	26.75	34.02
29.72	31.55	32.69	29.69	35.66	28.57	38.63	27.65	41.6	26.71
42.71	26.39	44.58	25.84	47.55	25.3	50.52	25.29	52.72	25.65
53.49	25.77	56.46	26.74	59.43	27.82	62.41	29.04	65.38	30.31
68.35	30.86	71.32	31.5	74.29	32.78	77.27	34.25	80.24	36.31
83.21	38.55	86.18	40.56	89.15	42.72	92.12	45.48	95.1	47.37
98.07	48.4	101.04	48.56	104.01	48.42				

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.08	44.58	.045	53.49	.08

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
44.58	53.49		173.88	173.98	174.09	.1	.3	

CROSS SECTION

RIVER: ChurchCrk

REACH: Reach1 RS: 1664

INPUT

Description:

Station Elevation Data num= 43

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	44.02	2.99	43.97	5.97	44.02	8.96	43.97	11.95	43.91
14.93	43.55	17.92	41.95	20.9	39.39	23.89	36.96	26.88	34.88
29.86	33.65	32.85	32.29	35.84	30.21	38.82	28.06	41.81	25.58
43.57	24.47	44.79	23.69	47.78	23	50.77	23.25	53.75	24.05
53.85	24.07	56.74	24.82	59.73	25.69	62.71	26.81	65.7	28.4
68.68	29.99	71.67	31.32	74.66	32.54	77.64	33.75	80.63	34.96
83.62	35.87	86.6	37.23	89.59	39.51	92.57	42.07	95.56	44.23
98.55	45.43	101.53	46.62	104.52	47.94	107.51	49.28	110.49	49.87
113.48	49.62	116.46	49.36	119.45	49.19				

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.08	44.79	.045	50.77	.08

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
44.79	50.77		173.7	175.5	177.29	.1	.3	

CROSS SECTION

RIVER: ChurchCrk

REACH: Reach1 RS: 1489

INPUT

Description:

Station Elevation Data num= 63

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	35.06	2.93	34.98	5.85	34.81	8.78	34.67	11.71	34.57
14.63	34.42	17.56	34.24	20.49	34.13	23.41	34.09	26.34	34.03
29.27	33.96	32.19	33.9	35.12	33.81	38.05	33.68	40.97	33.51
43.9	33.34	46.83	33.15	49.75	32.92	52.68	32.7	55.6	32.49
58.53	32.3	61.46	32.07	64.38	31.67	67.31	31.14	70.3	29.96
73.28	28.19	76.27	26.41	79.25	25.11	82.24	23.99	85.23	22.56
88.21	21.21	90.29	20.64	91.2	20.4	94.18	20.43	97.17	20.69
100.16	22.09	100.31	22.2	103.14	24.11	106.13	25.98	109.11	27.4
112.1	28.54	115.08	29.45	118.07	30.19	121.06	30.87	124.04	31.37
127.03	31.69	130.01	31.95	133	32.28	135.99	32.88	138.97	33.75
141.96	34.9	144.94	35.8	147.93	36.39	150.92	37.23	153.9	38.32
156.89	39.55	159.87	40.89	162.86	41.9	165.84	42.88	168.83	43.84
171.82	44.88	174.8	46.03	177.79	47.12				

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.08	88.21	.045	97.17	.08

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	88.21	97.17	224.06	226.16	226.11	.1	.3	

CROSS SECTION

RIVER: ChurchCrk

REACH: Reach1 RS: 1263

INPUT

Description:

Station Elevation Data num= 126

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	30	2.94	29.67	5.89	29.34	8.83	29.01	11.77	28.68
14.71	28.35	17.66	28.02	20.6	27.69	23.54	27.36	26.49	27.03
29.43	26.7	32.37	26.37	35.31	26.04	38.26	25.71	41.2	25.38
44.14	25.05	47.08	24.79	50.03	24.65	52.97	24.68	55.91	24.67
58.86	24.46	61.8	24.29	64.74	24.11	67.68	23.9	70.63	23.83
73.57	23.82	76.51	23.76	79.46	23.66	82.4	23.5	85.34	23.35
88.28	23.2	91.23	23.12	94.17	23.06	97.11	23.12	100.06	22.94
103	22.49	105.94	21.95	108.83	21.49	111.71	20.95	114.6	20.41
117.48	19.8	120.36	19.01	123.25	18.52	126.13	18.15	129.02	17.23
131.9	17.06	133.34	17.1	134.79	17.15	137.67	17.25	140.56	17.33
143.42	17.42	143.44	17.42	146.33	18	149.23	19.43	152.12	19.52
155.01	19.23	157.9	18.89	160.8	18.75	163.69	18.69	166.58	18.65
169.48	18.62	172.37	18.56	175.26	18.54	178.15	18.56	181.05	18.76
183.94	18.87	186.83	18.74	189.73	18.62	192.62	18.38	195.51	18.29
198.4	17.95	201.3	17.97	204.19	18.35	207.08	18.79	209.98	19.11
212.87	19.34	215.76	19.56	218.65	19.77	221.55	20.01	224.49	20.22
227.43	20.75	230.37	21.16	233.32	21.25	236.26	21.38	239.2	21.6
242.15	21.8	245.09	22.18	248.03	22.65	250.97	23.19	253.92	23.82

256.86	23.84	259.8	23.83	262.74	23.92	265.69	24.16	268.63	24.51
271.57	24.92	274.51	25.31	277.46	25.67	280.4	26.1	283.34	26.52
286.28	26.94	289.23	27.24	292.17	27.5	295.11	27.81	298.05	28.17
301	28.58	303.94	29.02	306.88	29.79	309.83	30.24	312.77	30.4
315.71	30.64	318.65	31.01	321.6	31.31	324.54	31.53	327.48	31.87
330.42	32.24	333.37	32.7	336.31	33.19	339.25	33.67	342.19	34.15
345.14	34.43	348.08	34.57	351.02	34.93	353.96	35.52	356.91	35.78
359.85	36.01								

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .08 129.02 .045 143.44 .08

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 129.02 143.44 210.32 320.95 333.63 .1 .3

CROSS SECTION

RIVER: ChurchCrk
 REACH: Reach1 RS: 942

INPUT

Description:

Station Elevation Data num= 90

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	28.08	2.97	28.12	5.93	28.28	8.9	28.39	11.87	28.32
14.83	27.89	17.8	27.68	20.76	27.43	23.73	26.8	26.7	26.35
29.66	25.98	32.63	25.64	35.6	25.49	38.56	25.4	41.53	25.06
44.5	24.74	47.46	24.51	50.43	23.97	53.39	23.36	56.36	22.68
59.33	22.06	62.29	21.34	65.26	20.47	68.23	19.56	71.19	18.71
74.16	17.86	77.13	17.11	80.09	16.8	83.06	16.31	86.02	16.16
88.99	15.98	91.96	15.76	94.92	15.49	97.89	15.34	100.86	15.22
103.82	15.12	106.79	15.09	109.76	15.12	112.72	15.18	115.69	15.21
118.65	15.11	121.62	14.93	124.59	14.47	124.61	14.47	127.55	14.12
130.52	14	133.49	14.02	134.69	14.1	136.45	14.21	139.42	14.35
142.39	14.45	145.35	14.71	148.32	15.16	151.28	15.51	154.25	15.57
157.22	15.55	160.18	15.81	163.15	15.52	166.12	15.48	169.08	15.58
172.05	15.58	175.01	15.67	177.98	15.77	180.95	16.06	183.91	16.56
186.88	17.36	189.85	18.33	192.81	19.85	195.78	21.44	198.75	22.77
201.71	23.64	204.68	24.18	207.65	24.74	210.61	25.27	213.58	25.78
216.54	26.17	219.51	26.47	222.48	26.64	225.44	26.91	228.41	27.17
231.38	27.19	234.34	27.22	237.31	27.22	240.28	27.53	243.24	27.64
246.21	27.83	249.17	27.96	252.14	28.05	255.11	28.11	258.07	28

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .08 127.55 .045 139.42 .08

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 127.55 139.42 254.95 241.84 232.76 .1 .3

CROSS SECTION

RIVER: ChurchCrk
REACH: Reach1 RS: 700

INPUT

Description:

Station Elevation Data num= 37

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	24.39	2.92	23.57	5.84	22.71	8.76	21.79	11.69	21.11
14.61	20.57	17.53	20.15	20.45	19.78	23.37	19.18	26.29	18.46
29.21	17.67	32.13	16.43	35.06	15.18	37.98	14.08	40.9	13.34
40.95	13.34	43.82	13.2	46.74	13.26	49.66	13.33	50.98	13.47
52.58	13.65	55.51	14.23	58.43	14.63	61.35	14.85	64.27	15.41
67.19	16.51	70.11	17.72	73.03	19.55	75.95	21.49	78.88	22.77
81.8	23.67	84.72	24.21	87.64	24.82	90.56	25.7	93.48	26.87
96.4	27.76	99.32	28.41						

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.08	40.95	.045	50.98	.08

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	40.95	50.98		36.13	45.54	54.7	.1	.3

CROSS SECTION

RIVER: ChurchCrk
REACH: Reach1 RS: 654

INPUT

Description:

Station Elevation Data num= 45

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	29.99	2.75	29.21	5.49	28.62	8.24	28.13	10.98	27.64
13.73	27.16	16.47	26.78	19.22	26.1	21.96	25.08	24.71	24.05
27.64	23.2	30.56	22.81	33.49	22.7	36.42	22.34	39.35	22.03
42.28	21.63	45.21	21.28	48.14	20.88	51.07	20.05	54	19.5
56.93	19.03	57.6	18.96	59.86	10.56	62.79	10.56	65.72	10.56
67.81	10.56	68.64	10.56	71.57	19.21	74.5	20.38	77.43	21.47
80.36	22.9	83.29	24.27	86.22	25.68	89.15	27.08	92.08	28.42
95.01	29.73	97.94	31.15	100.87	32.32	103.79	33.04	106.72	33.66
109.65	33.92	112.58	33.94	115.51	33.68	118.44	33.16	121.37	32.7

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.08	57.6	.045	71.57	.08

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	57.6	71.57		217.43	223.71	229.88	.1	.3

CULVERT

RIVER: ChurchCrk
REACH: Reach1 RS: 595

INPUT

Description:

Distance from Upstream XS = 60

Deck/Roadway Width = 105

Weir Coefficient = 2.6

Upstream Deck/Roadway Coordinates

num= 2

Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord
0	33	150	33		

Upstream Bridge Cross Section Data

Station Elevation Data num= 45

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	29.99	2.75	29.21	5.49	28.62	8.24	28.13	10.98	27.64
13.73	27.16	16.47	26.78	19.22	26.1	21.96	25.08	24.71	24.05
27.64	23.2	30.56	22.81	33.49	22.7	36.42	22.34	39.35	22.03
42.28	21.63	45.21	21.28	48.14	20.88	51.07	20.05	54	19.5
56.93	19.03	57.6	18.96	59.86	10.56	62.79	10.56	65.72	10.56
67.81	10.56	68.64	10.56	71.57	19.21	74.5	20.38	77.43	21.47
80.36	22.9	83.29	24.27	86.22	25.68	89.15	27.08	92.08	28.42
95.01	29.73	97.94	31.15	100.87	32.32	103.79	33.04	106.72	33.66
109.65	33.92	112.58	33.94	115.51	33.68	118.44	33.16	121.37	32.7

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.08	57.6	.045	71.57	.08

Bank Sta: Left Right Coeff Contr. Expan.

57.6	71.57	.1	.3
------	-------	----	----

Downstream Deck/Roadway Coordinates

num= 2

Sta	Hi Cord	Lo Cord	Sta	Hi Cord	Lo Cord
0	33	250	33		

Downstream Bridge Cross Section Data

Station Elevation Data num= 80

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	31.16	2.98	31.26	5.95	31.23	8.93	31.12	11.9	30.76
14.88	30.41	17.85	29.75	20.83	29.21	23.8	27.89	26.78	25.96
29.75	23.69	32.73	21.22	35.7	18.73	38.68	16.45	41.65	14.89
44.63	13.9	47.6	13.77	50.58	13.69	53.55	13.31	56.53	12.88
59.5	9.56	62.48	9.56	62.87	9.56	65.46	9.56	68.43	9.56
71.41	12.93	72.87	13.21	74.38	13.5	77.36	14.07	80.33	14.52
83.31	15	86.27	15.41	89.23	15.79	92.19	16.04	95.15	16.29
98.11	16.49	101.07	16.63	104.03	16.61	106.99	16.75	109.95	16.98
112.91	16.97	115.87	16.76	118.83	16.68	121.79	16.65	124.75	16.6
127.71	17.28	130.67	18.25	133.63	19.41	136.59	20.49	139.55	21.06
142.51	21.54	145.47	22.11	148.43	22.73	151.39	23.34	154.35	23.96
157.31	24.54	160.27	25.1	163.23	25.74	166.19	26.48	169.15	27.28
172.11	28.06	175.07	28.64	178.03	29.4	180.99	30.1	183.95	31.14

186.91 32.31 189.87 33.14 192.83 33.69 195.79 33.84 198.75 33.63
 201.71 33.43 204.67 33.4 207.63 33.52 210.59 33.7 213.55 34.07
 216.51 34.32 219.47 34.38 222.43 34.5 225.39 34.74 228.35 35

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .08 56.53 .045 71.41 .08

Bank Sta: Left Right Coeff Contr. Expan.
 56.53 71.41 .1 .3

Upstream Embankment side slope = 0 horiz. to 1.0 vertical
 Downstream Embankment side slope = 0 horiz. to 1.0 vertical
 Maximum allowable submergence for weir flow = .98
 Elevation at which weir flow begins =
 Energy head used in spillway design =
 Spillway height used in design =
 Weir crest shape = Broad Crested

Number of Culverts = 1

Culvert Name Shape Rise Span
 Culvert #1 Circular 8
 FHWA Chart # 2 - Corrugated Metal Pipe Culvert
 FHWA Scale # 2 - Mitered to conform to slope
 Solution Criteria = Highest U.S. EG
 Culvert Upstrm Dist Length Top n Bottom n Depth Blocked Entrance Loss Coef Exit Loss Coef
 5 148 .024 .013 0 .7 1
 Upstream Elevation = 10.56
 Centerline Station = 65
 Downstream Elevation = 9.56
 Centerline Station = 65

CROSS SECTION

RIVER: ChurchCrk
 REACH: Reach1 RS: 431

INPUT

Description:

Station Elevation Data num= 80
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev
 0 31.16 2.98 31.26 5.95 31.23 8.93 31.12 11.9 30.76
 14.88 30.41 17.85 29.75 20.83 29.21 23.8 27.89 26.78 25.96
 29.75 23.69 32.73 21.22 35.7 18.73 38.68 16.45 41.65 14.89
 44.63 13.9 47.6 13.77 50.58 13.69 53.55 13.31 56.53 12.88
 59.5 9.56 62.48 9.56 62.87 9.56 65.46 9.56 68.43 9.56
 71.41 12.93 72.87 13.21 74.38 13.5 77.36 14.07 80.33 14.52
 83.31 15 86.27 15.41 89.23 15.79 92.19 16.04 95.15 16.29
 98.11 16.49 101.07 16.63 104.03 16.61 106.99 16.75 109.95 16.98
 112.91 16.97 115.87 16.76 118.83 16.68 121.79 16.65 124.75 16.6
 127.71 17.28 130.67 18.25 133.63 19.41 136.59 20.49 139.55 21.06
 142.51 21.54 145.47 22.11 148.43 22.73 151.39 23.34 154.35 23.96

157.31	24.54	160.27	25.1	163.23	25.74	166.19	26.48	169.15	27.28
172.11	28.06	175.07	28.64	178.03	29.4	180.99	30.1	183.95	31.14
186.91	32.31	189.87	33.14	192.83	33.69	195.79	33.84	198.75	33.63
201.71	33.43	204.67	33.4	207.63	33.52	210.59	33.7	213.55	34.07
216.51	34.32	219.47	34.38	222.43	34.5	225.39	34.74	228.35	35

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .08 56.53 .045 71.41 .08

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 56.53 71.41 137.98 136.16 134.34 .1 .3

CROSS SECTION

RIVER: ChurchCrk
 REACH: Reach1 RS: 295

INPUT
 Description:

Station Elevation Data num= 88

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	24.3	2.95	23.09	5.91	21.79	8.86	20.29	11.81	18.68
14.77	17.14	17.72	16.2	20.67	15.82	23.63	15.62	26.58	15.3
29.53	14.82	32.49	14.39	35.44	14.17	38.39	14.06	41.35	13.84
44.3	13.74	47.25	13.65	50.21	13.5	53.16	13.28	56.11	12.88
59.07	12.57	62.02	12.56	64.97	12.57	67.93	12.64	70.88	12.7
73.83	12.88	76.79	12.83	79.74	12.7	82.69	12.69	85.65	12.72
88.6	12.57	91.55	12.2	94.51	11.29	97.46	10.36	100.41	9.77
103.37	9.17	106.32	8.7	109.27	8.7	112.23	9.04	114.99	9.36
115.18	9.38	118.13	9.75	121.09	10.14	124.04	10.52	125.01	10.71
126.99	11.08	129.95	11.77	132.9	12.27	135.88	12.61	138.86	12.78
141.84	12.88	144.82	12.89	147.8	12.87	150.78	12.83	153.76	12.82
156.74	12.82	159.72	12.84	162.7	12.92	165.68	13.14	168.66	13.19
171.64	13.5	174.62	13.99	177.6	14.49	180.58	15.06	183.56	15.59
186.54	15.72	189.52	15.37	192.5	14.89	195.48	15.1	198.46	15.49
201.45	15.9	204.43	16.49	207.41	17.57	210.39	19.44	213.37	21.39
216.35	22.23	219.33	22.68	222.31	22.85	225.29	22.94	228.27	23.23
231.25	23.47	234.23	23.63	237.21	24.09	240.19	24.81	243.17	25.63
246.15	27.18	249.13	28.83	252.11	30.27				

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val
 0 .08 100.41 .045 118.13 .08

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.
 100.41 118.13 209.15 202.38 195.36 .1 .3

CROSS SECTION

RIVER: ChurchCrk
 REACH: Reach1 RS: 92

INPUT

Description:

Station Elevation Data num= 88

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	13.81	2.97	13.25	5.94	12.7	8.91	12.23	11.88	11.87
14.85	11.57	17.83	11.08	20.8	10.64	23.77	10.38	26.74	10.14
29.71	10.16	32.68	10.27	35.65	10.17	38.62	9.86	41.59	9.46
44.56	9.32	47.53	9.4	50.51	9.53	53.48	9.4	56.45	9.58
59.42	9.43	62.39	9.3	65.36	9.09	68.33	8.96	71.3	9.05
74.27	9.06	77.24	9.07	80.21	9.06	83.19	9.03	86.16	9.03
89.13	9.12	92.1	9.28	95.07	9.35	98.04	9.36	101.01	9.43
103.98	9.6	106.95	9.74	109.92	9.77	112.89	9.81	115.87	9.81
118.84	9.75	121.81	9.63	124.78	9.81	127.75	10.07	130.72	10.14
133.69	9.89	136.66	9.64	139.63	9.51	142.6	9.58	145.57	9.57
148.55	9.46	151.52	9.3	154.49	9.25	157.46	9.22	160.43	9.2
163.4	9.1	166.37	9.07	169.34	8.92	172.31	8.31	175.28	7.78
178.25	7.48	179.03	7.52	181.23	7.62	184.2	8.19	187.17	8.35
190.14	8.61	191.16	8.68	193.11	8.83	196.08	8.92	199.05	8.8
202.02	8.73	204.99	8.81	207.96	8.9	210.93	9	213.91	9.14
216.88	9.2	219.85	9.18	222.82	9.15	225.79	9.31	228.76	9.52
231.73	9.91	234.7	10.35	237.67	10.74	240.64	11.1	243.62	11.36
246.59	11.89	249.56	13	252.53	13.91				

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
0	.08	169.34	.045	191.16	.08

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	169.34	191.16	92.94	92.25	91.79	.1	.3	

SUMMARY OF MANNING'S N VALUES

River:ChurchCrk

Reach	River Sta.	n1	n2	n3
Reach1	2442	.08	.045	.08
Reach1	2210	.08	.045	.08
Reach1	1838	.08	.045	.08
Reach1	1664	.08	.045	.08
Reach1	1489	.08	.045	.08
Reach1	1263	.08	.045	.08
Reach1	942	.08	.045	.08
Reach1	700	.08	.045	.08
Reach1	654	.08	.045	.08
Reach1	595	Culvert		
Reach1	431	.08	.045	.08
Reach1	295	.08	.045	.08
Reach1	92	.08	.045	.08

SUMMARY OF REACH LENGTHS

River: ChurchCrk

Reach	River Sta.	Left	Channel	Right
Reach1	2442	225.53	232.08	237.13
Reach1	2210	376.59	371.84	368.59
Reach1	1838	173.88	173.98	174.09
Reach1	1664	173.7	175.5	177.29
Reach1	1489	224.06	226.16	226.11
Reach1	1263	210.32	320.95	333.63
Reach1	942	254.95	241.84	232.76
Reach1	700	36.13	45.54	54.7
Reach1	654	217.43	223.71	229.88
Reach1	595	Culvert		
Reach1	431	137.98	136.16	134.34
Reach1	295	209.15	202.38	195.36
Reach1	92	92.94	92.25	91.79

SUMMARY OF CONTRACTION AND EXPANSION COEFFICIENTS

River: ChurchCrk

Reach	River Sta.	Contr.	Expan.
Reach1	2442	.1	.3
Reach1	2210	.1	.3
Reach1	1838	.1	.3
Reach1	1664	.1	.3
Reach1	1489	.1	.3
Reach1	1263	.1	.3
Reach1	942	.1	.3
Reach1	700	.1	.3
Reach1	654	.1	.3
Reach1	595	Culvert	
Reach1	431	.1	.3
Reach1	295	.1	.3
Reach1	92	.1	.3

HEC-RAS Version 4.1.0 Jan 2010
 U.S. Army Corps of Engineers
 Hydrologic Engineering Center
 609 Second Street
 Davis, California

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X  X XXXXXX  XXXX  XXXX  XX  XXXX
X  X X  X X  X X  X X  X
X  X X  X  X X  X X  X
XXXXXXXX XXXX  X  XXX XXXX  XXXXXX  XXXX
X  X X  X  X X  X X  X
X  X X  X X  X X  X X  X
X  X XXXXXX  XXXX  X  X X  X XXXXX
  
```

PROJECT DATA

Project Title: ChurchCreek
 Project File : ChurchCreek.prj
 Run Date and Time: 10/31/2017 1:22:43 PM

Project in English units

Profile Output Table - Standard Table 1

Reach Chnl	River Sta Flow Area	Profile Top Width	Froude #	Q Total Chl	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft/ft)	E.G. Elev (sq ft)	E.G. Slope (ft)	Vel
			(cfs)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)
Reach1 30.52	2442 20.85	1 year 24 hour 0.94	183.00000	31.49	33.96	33.96	34.82	0.020560	7.96	
Reach1 39.14	2442 23.73	2 year 24 hou 0.96	247.00000	31.49	34.35	34.35	35.36	0.020288	8.80	
Reach1 61.23	2442 35.95	10 year 24 hour 0.93	371.00000	31.49	35.10	35.10	36.24	0.017371	9.64	
Reach1 98.51	2442 65.55	100 year 24 hour 0.88	521.00000	31.49	35.86	35.86	37.03	0.014606	10.14	
Reach1 92.02	2210 71.13	1 year 24 hour 0.41	183.00000	27.47	30.37		30.51	0.003581	3.74	
Reach1 127.09	2210 74.08	2 year 24 hou 0.37	247.00000	27.47	30.85		30.98	0.002799	3.70	
Reach1 411.84	2210 90.14	10 year 24 hour 0.12	371.00000	27.47	34.31		34.33	0.000220	1.71	
Reach1 502.36	2210 93.60	100 year 24 hour 0.13	521.00000	27.47	35.29		35.32	0.000243	1.97	
Reach1 57.85	1838 26.86	1 year 24 hour 0.45	183.00000	25.29	28.82		29.09	0.004071	4.69	
Reach1	1838	2 year 24 hou	247.00000	25.29	29.52		29.81	0.003476	4.92	

78.11	30.41	0.43							
Reach1	1838	10 year 24 hour	371.00000	25.29	34.16		34.22	0.000336	2.54
271.14	50.52	0.15							
Reach1	1838	100 year 24 hour	521.00000	25.29	35.11		35.20	0.000426	3.07
320.25	53.09	0.17							
Reach1	1664	1 year 24 hour	280.00000	23.00	26.72	26.48	27.64	0.014549	9.07
49.68	22.05	0.86							
Reach1	1664	2 year 24 hou	374.00000	23.00	27.01	27.01	28.30	0.018895	10.90
56.14	23.00	0.99							
Reach1	1664	10 year 24 hour	558.00000	23.00	34.02		34.14	0.000583	3.85
301.63	49.34	0.21							
Reach1	1664	100 year 24 hour	781.00000	23.00	34.90		35.08	0.000813	4.80
347.10	53.64	0.25							
Reach1	1489	1 year 24 hour	280.00000	20.40	23.48	23.48	24.66	0.019666	9.31
39.17	18.90	0.97							
Reach1	1489	2 year 24 hou	374.00000	20.40	26.28		26.63	0.002700	5.42
107.71	30.20	0.40							
Reach1	1489	10 year 24 hour	558.00000	20.40	34.01		34.07	0.000180	2.48
537.28	112.57	0.12							
Reach1	1489	100 year 24 hour	781.00000	20.40	34.90		34.98	0.000248	3.04
650.04	137.67	0.14							
Reach1	1263	1 year 24 hour	280.00000	17.06	21.77		21.79	0.000308	1.59
362.00	134.59	0.13							
Reach1	1263	2 year 24 hou	374.00000	17.06	26.51		26.51	0.000021	0.66
1283.71	252.12	0.04							
Reach1	1263	10 year 24 hour	558.00000	17.06	34.05		34.05	0.000003	0.35
3591.62	341.56	0.02							
Reach1	1263	100 year 24 hour	781.00000	17.06	34.95		34.95	0.000004	0.46
3904.33	351.12	0.02							
Reach1	942	1 year 24 hour	539.00000	14.00	21.73		21.74	0.000119	1.39
759.95	135.72	0.09							
Reach1	942	2 year 24 hou	723.00000	14.00	26.50		26.50	0.000035	1.04
1517.05	194.23	0.05							
Reach1	942	10 year 24 hour	1247.00000	14.00	34.04		34.05	0.000012	0.83
3393.72	258.07	0.03							
Reach1	942	100 year 24 hour	1833.00000	14.00	34.94		34.95	0.000020	1.13
3625.69	258.07	0.04							
Reach1	700	1 year 24 hour	539.00000	13.20	21.59		21.68	0.000534	3.13
329.33	66.58	0.19							
Reach1	700	2 year 24 hou	723.00000	13.20	26.45		26.48	0.000130	2.10
724.23	92.44	0.10							
Reach1	700	10 year 24 hour	1247.00000	13.20	34.02		34.04	0.000053	1.81
1468.19	99.32	0.07							
Reach1	700	100 year 24 hour	1833.00000	13.20	34.90		34.94	0.000097	2.52
1555.30	99.32	0.10							
Reach1	654	1 year 24 hour	548.00000	10.56	21.36	15.23	21.62	0.001890	4.11
150.58	32.65	0.24							
Reach1	654	2 year 24 hou	734.00000	10.56	26.39	16.15	26.47	0.000426	2.61

